

# Seismic Hazard Assessment: Problems with Current Practice and Future Developments

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# Seismology

Complex, detailed models of fault geometries,  
Seismic source, wave propagation



# Engineering Seismology

Simplify seismology into statistical descriptions,  
Reduce to a few scenarios (PSHA)  
Develop representative ground motions



# Engineering

Consider only a few earthquake scenarios  
Complex models of structures

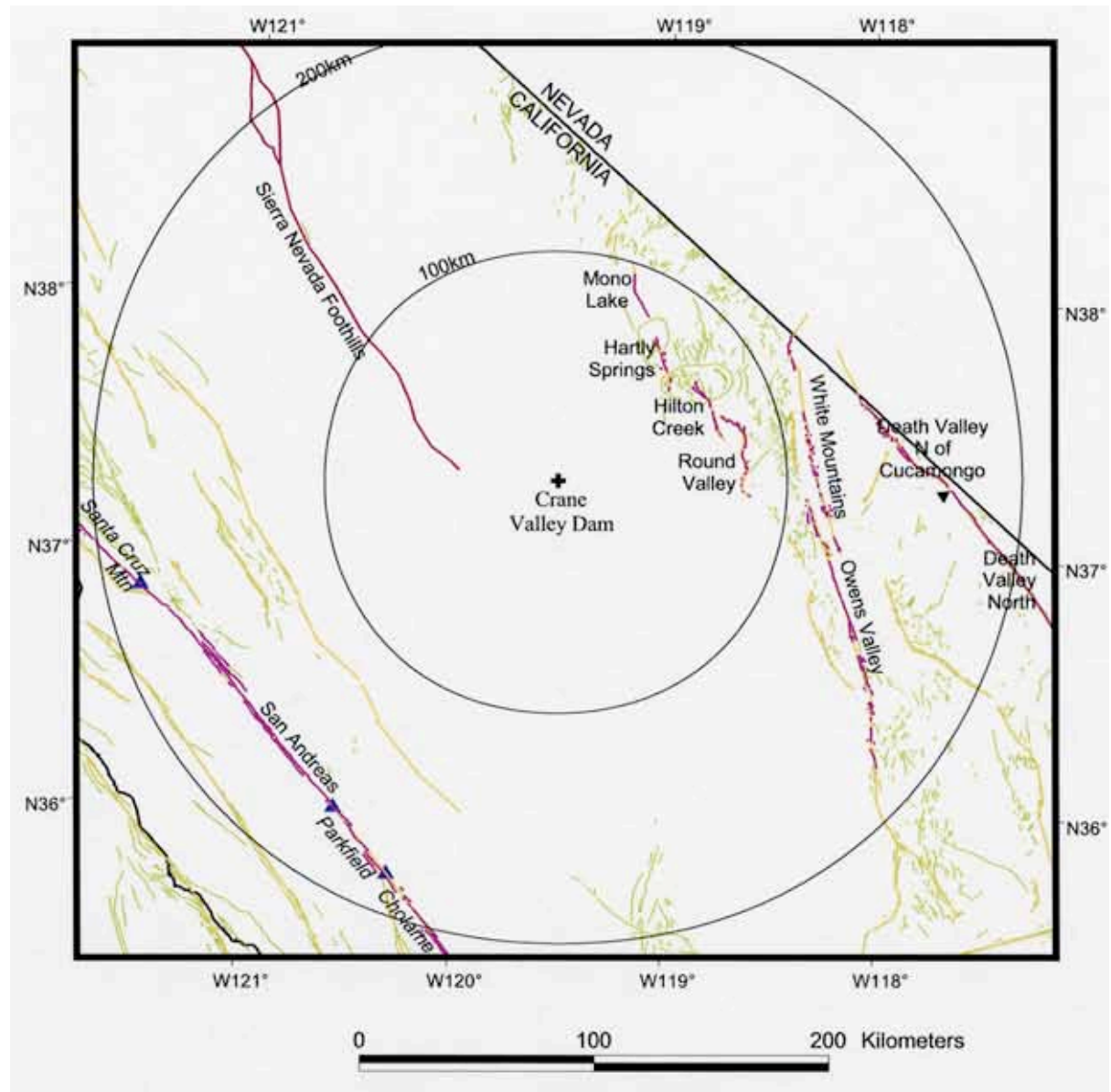
# Example: Dam Site in Eastern CA

**Design Earthquakes:**  
San Andreas Flt

$M_{\max} = 8,$

Distance = 170 km

Also, consider other flts



Need to  
consider  
earthquakes  
not on faults

**Design Earthquakes:**  
San Andreas Flt

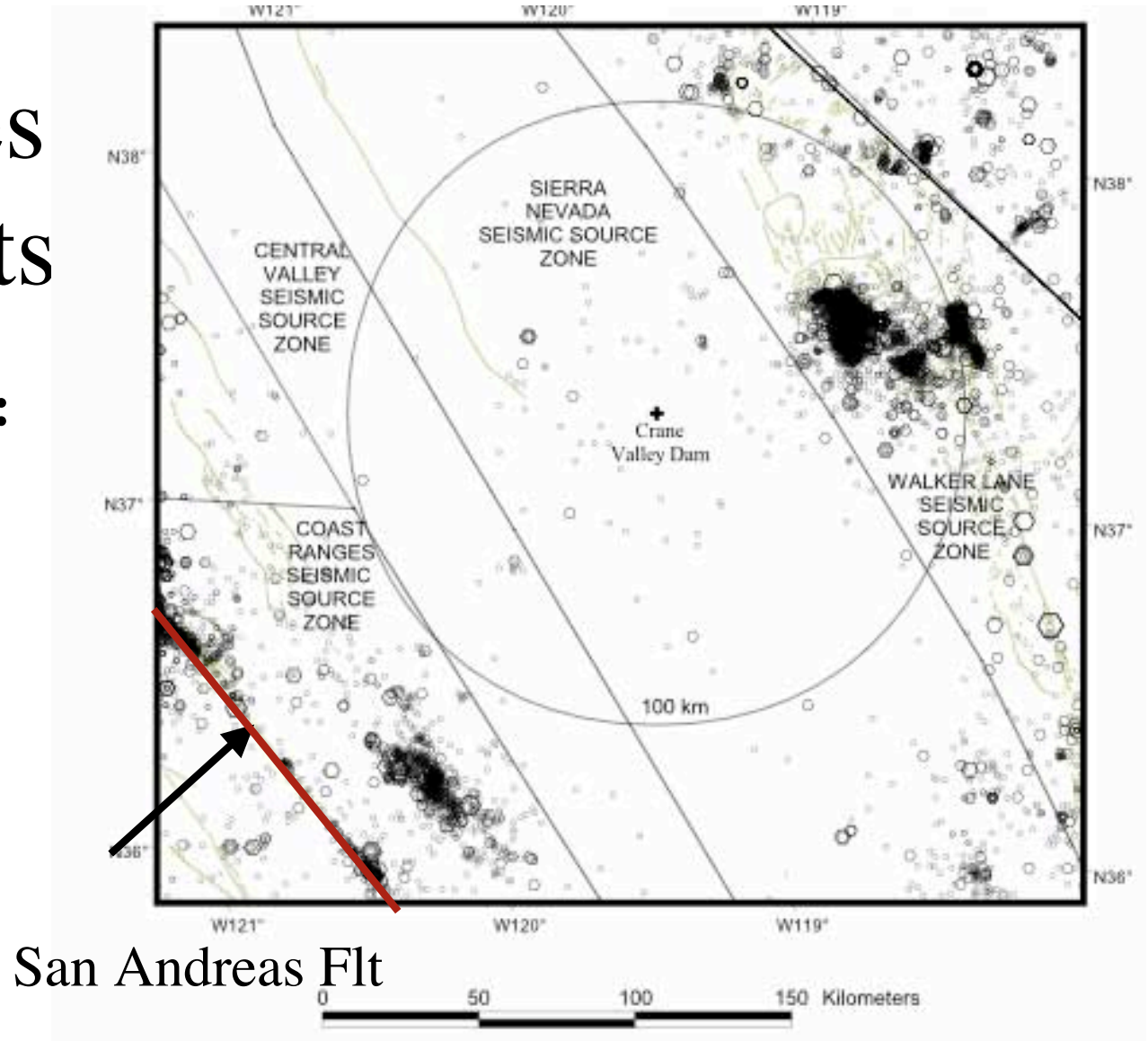
$M_{\max} = 8$ ,

Distance = 170 km

Sierra Nevada Zone

$M_{\max} = 6.5$

Distance = ?



# Background Earthquakes

- What should we use for design?
- Option 1: treat same as faults
  - Largest magnitude at closest location
    - $M=6.5$ , Distance = 0
    - Not “reasonable”
- Option 2: pick some less severe earthquake
  - $M=5.5$ , distance=5 km?
  - $M=6.0$ , distance=10 km?
  - $M=6.25$ , distance = 17 km?
  - What is reasonable? - Depends on seismic activity
- Difficulties in selecting a “reasonable” background earthquake is what lead to the development of PSHA

# Deterministic vs Probabilistic

- Deterministic
  - Consider of small number of scenarios (Mag, dist, number of standard deviation of ground motion)
  - Choose the largest ground motion from cases considered
- Probabilistic
  - Consider all possible scenarios: all mag-dist combinations, and number of std dev of ground motion ( $\epsilon$ )
  - Compute the rate of each scenario ( $M, R, \epsilon$ )
  - Identify the subset of ( $M, R, \epsilon$ ) scenarios with ground motion above a some threshold (e.g.  $S_a(T=1 \text{ sec})$  of  $0.5g$ )
  - Sum the rates of the scenarios in the subset of to determine rate of “exceedance”

# Terminology

- Aleatory Variability (random)
  - Randomness in M, location, ground motion ( $\epsilon$ )
- Incorporated in hazard calculation directly  
Epistemic Uncertainty (scientific)
  - Due to lack of information
  - Incorporated in PSHA using logic trees (leads to alternative hazard curves)

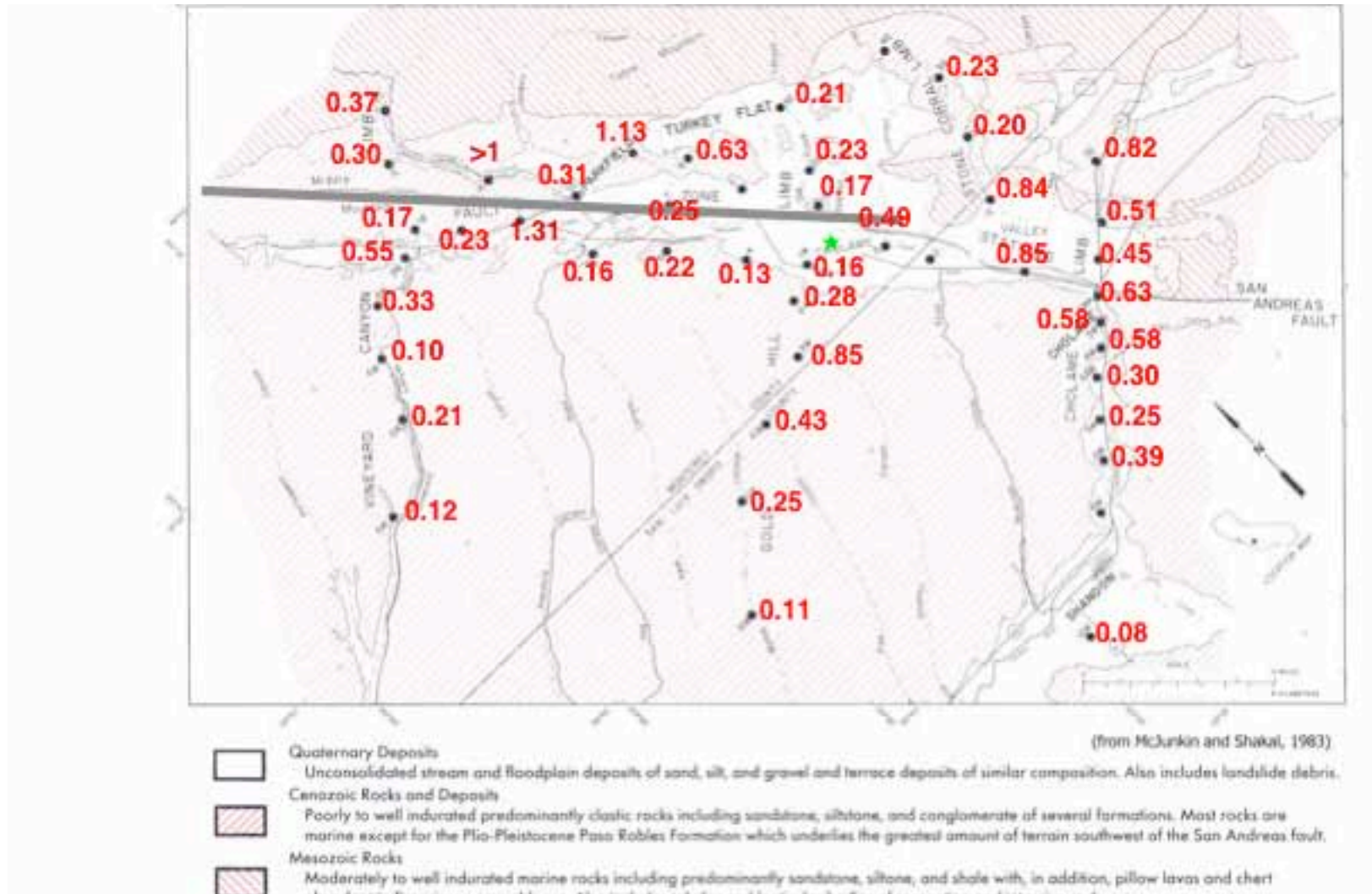
# Deterministic Approach

- Select a specific magnitude and distance (location) for each source
  - Typically, use the largest earthquake at the closest distance (except for background zone with site)
- Design for ground motion, not earthquakes
  - Ground motion has large variability for a given magnitude, distance, and site condition
  - Key issue: What ground motion level do we select?

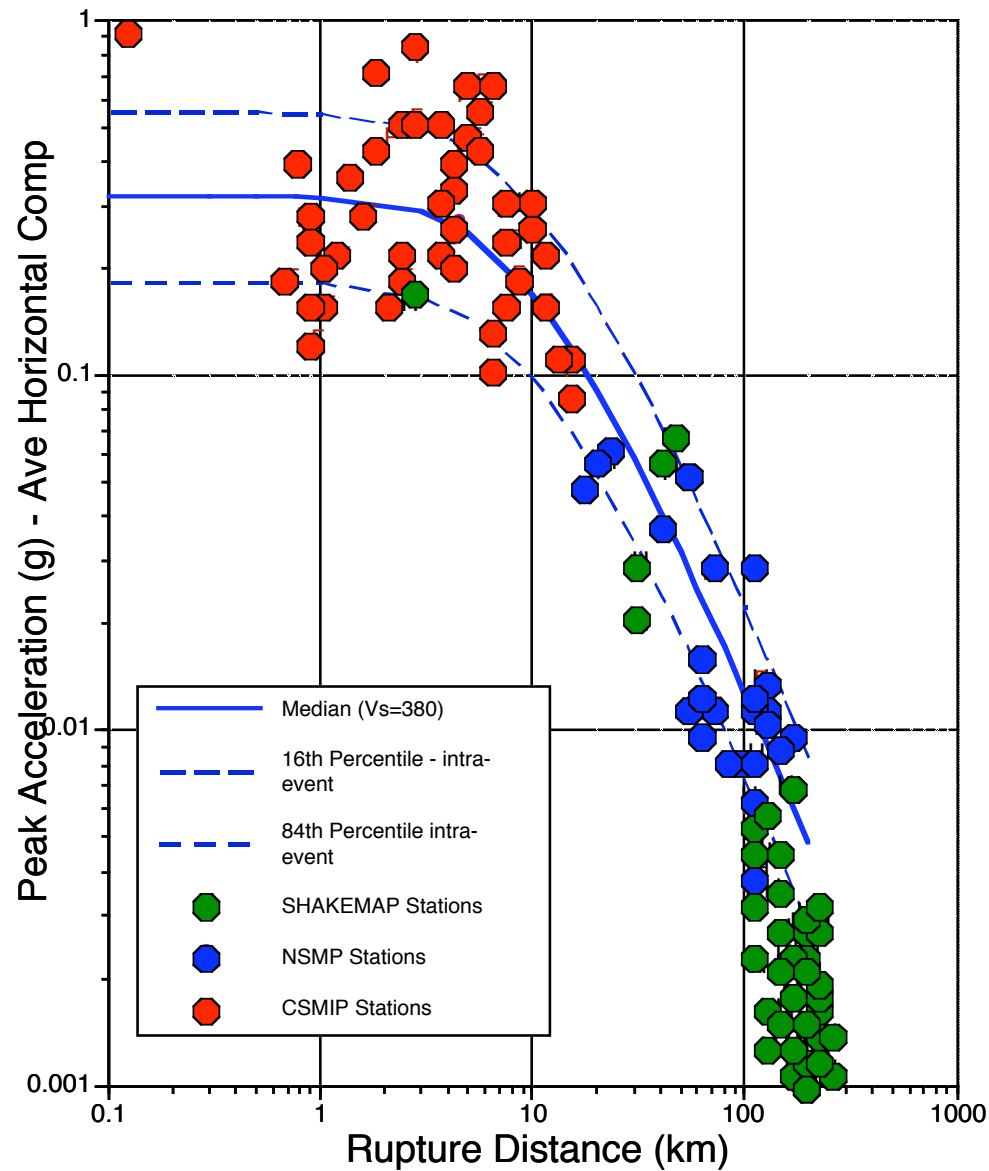


# 2004 Parkfield

## Near Fault PGA Values

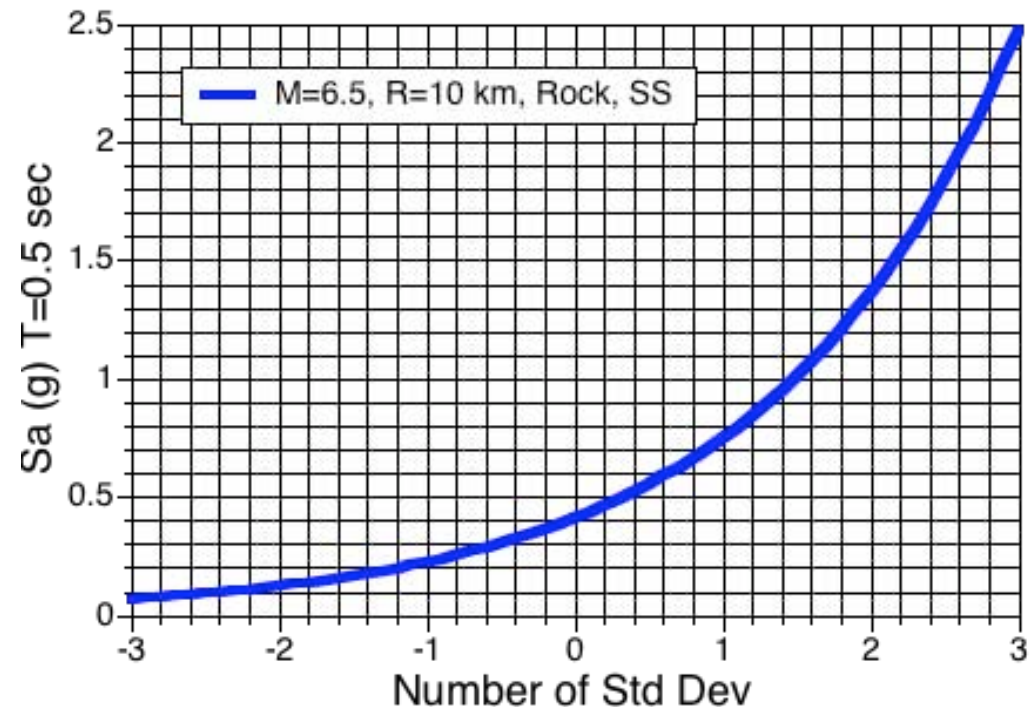


# 2004 Parkfield PGA Attenuation



# Ground Motion Levels

- By tradition, select median or 84th percentile
- Worst-case ground motion is much higher



# Worst-Case Ground Motion is Not Selected in Deterministic Approach

- Combining largest earthquake with the worst-case ground motion is too unlikely a case
  - The occurrence of the maximum earthquake is rare, so it is not “reasonable” to use a worst-case ground motion for this earthquake
  - Chose something smaller than the worst-case ground motion that is “reasonable”.

# What is “Reasonable”

- The same number of standard deviation of ground motion may not be “reasonable” for all sources
  - Median may be reasonable for low activity sources, but higher value may be needed for high activity sources
- Need to consider both the rate of the earthquake and the chance of the ground motion

# Probabilistic Approach

- Source Characterization
  - Develop a comprehensive set of possible scenario earthquakes:  $M$ ,  $R(\text{location})$
  - Specify the rate at which each scenario earthquake  $(M,R)$  occurs
- Ground Motion Characterization
  - Develop a full range of possible ground motions for each earthquake scenario ( $\epsilon$ =number of std dev above or below the median)
  - Compute the probability of each ground motion for each scenario

# Probabilistic Approach (cont)

- Hazard Calculation
  - Rank scenarios (M,R,  $\epsilon$ ) in order of decreasing severity of shaking (Here, use  $S_a$ )
  - Result: Table of ranked scenarios with ground motions and rates
  - Sum up rates of scenarios with ground motion above a specified level (hazard curve)
- Select a ground motion for the design hazard level
  - Back off from worst case ground motion until either:
    - The ground motion is does not lead to excessive costs, or
    - The hazard level is not too small (e.g. not too rare) to ignore (e.g. the design hazard level)
    - “Not too rare” hazard should be determined based on risk calculations

# Use of PHSA

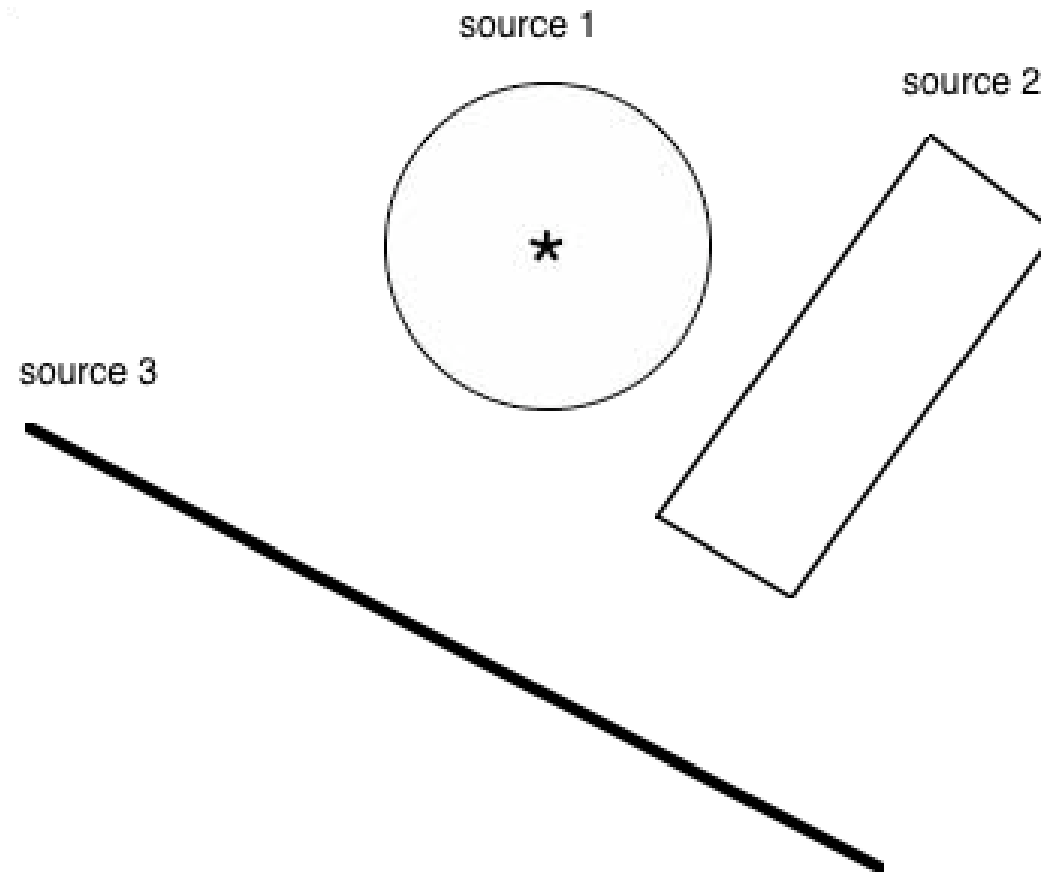
- Input for probabilistic risk calculation
  - Uses the full hazard curve (and deaggregation)
- Selection of design ground motion
  - Purpose of PHSA is to provide a method to select “reasonable” deterministic earthquake scenarios ( $M$ ,  $R$ ,  $\epsilon$ ) from the complete set of all scenarios
  - Select the most severe scenarios that is either not too rare or not too costly



# Design Ground Motions

- Not using worst-case ground motions
  - “reasonable” ground motions should have acceptably small risk of failure
- Risk calculation
  - Direct computation of probability of failure for a given design
- Probabilistic
  - Selection of a return period for design ground motions is a simplified risk calculation.
    - If we design for certain return period, then the probability of failure is smaller than the hazard
      - How much smaller? Factor of 2? 10?
- Deterministic
  - Simplified PHSA
    - If we choose a deterministic event and ground motion level, then assume that the probability of exceeding the ground motion is small enough to lead to acceptable risk

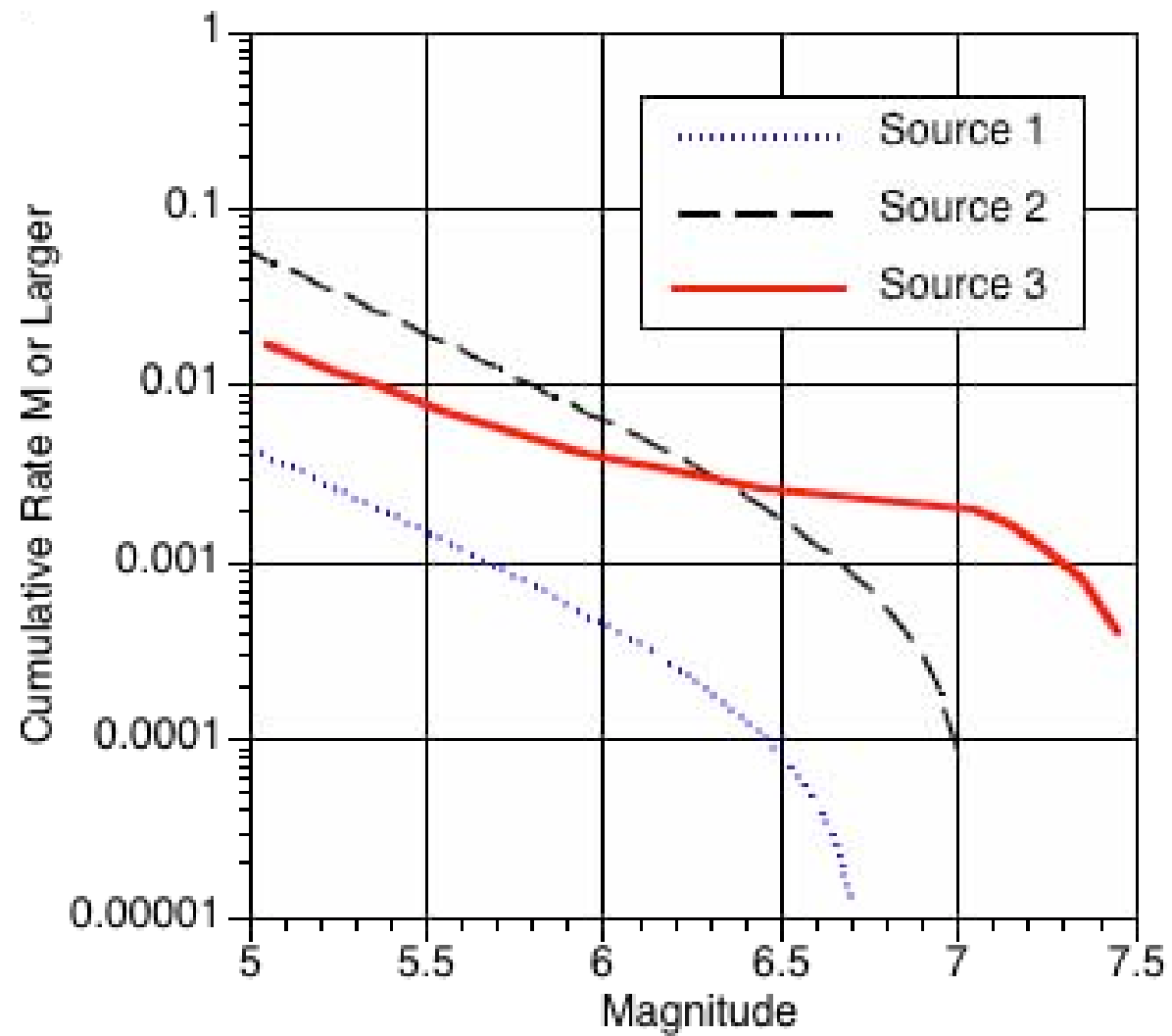
# Example Hazard Calculation w/o equations



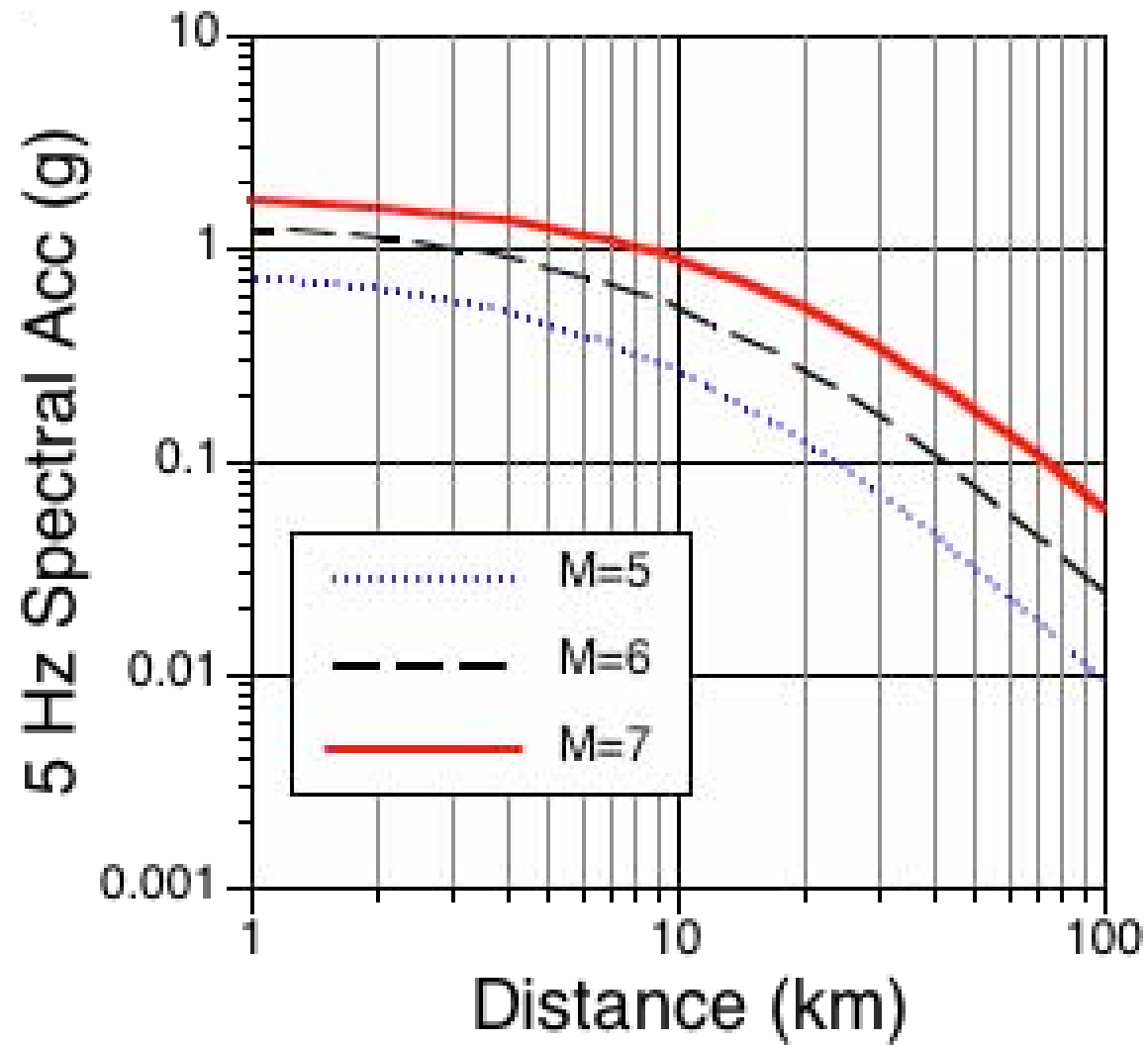
# Terminology

- Recurrence Interval
  - Refers to earthquakes on specific sources
  - From magnitude-recurrence curve
  - $1/(\text{Rate of given } M \text{ or larger})$
- Return Period
  - Refers to the ground motion at a specific site
  - From hazard curve
  - $1/(\text{Rate of given } S_a \text{ or larger})$

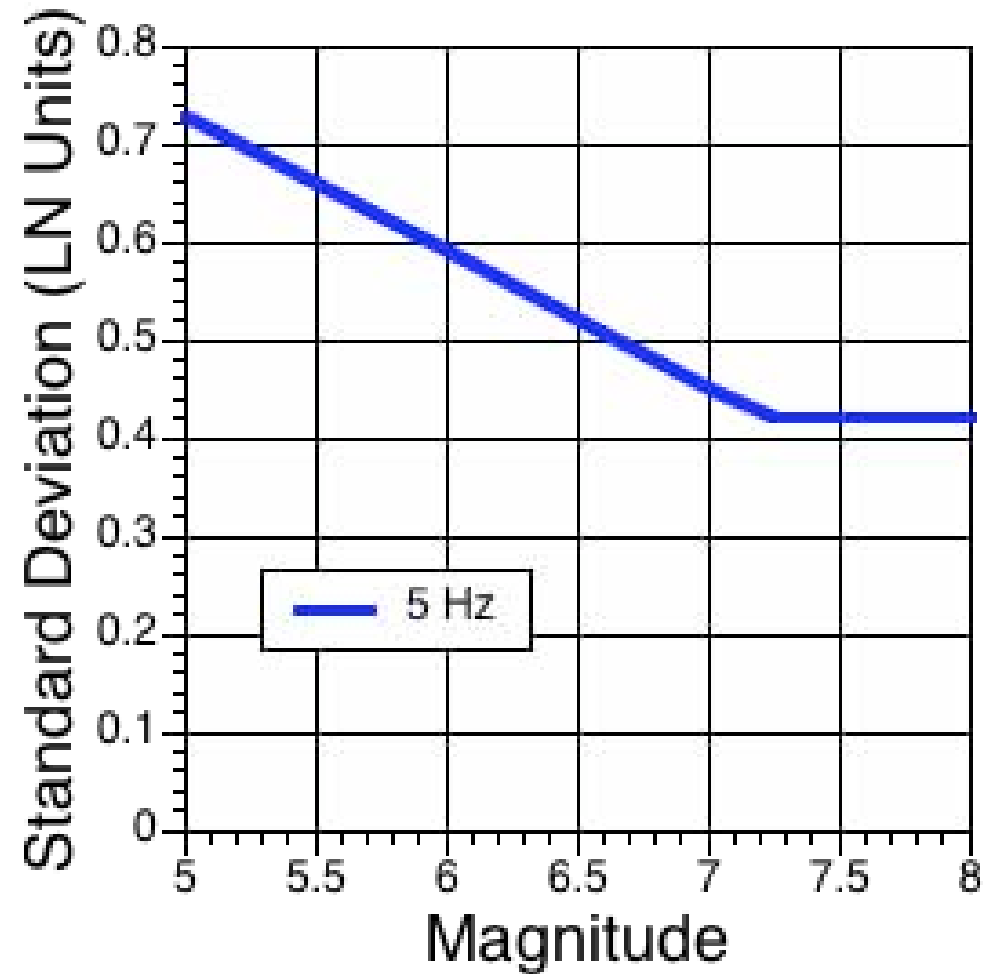
## Example Source Characterization



# Median Ground Motion



# Standard Deviation of Ground Motion



## Partial List of Scenarios

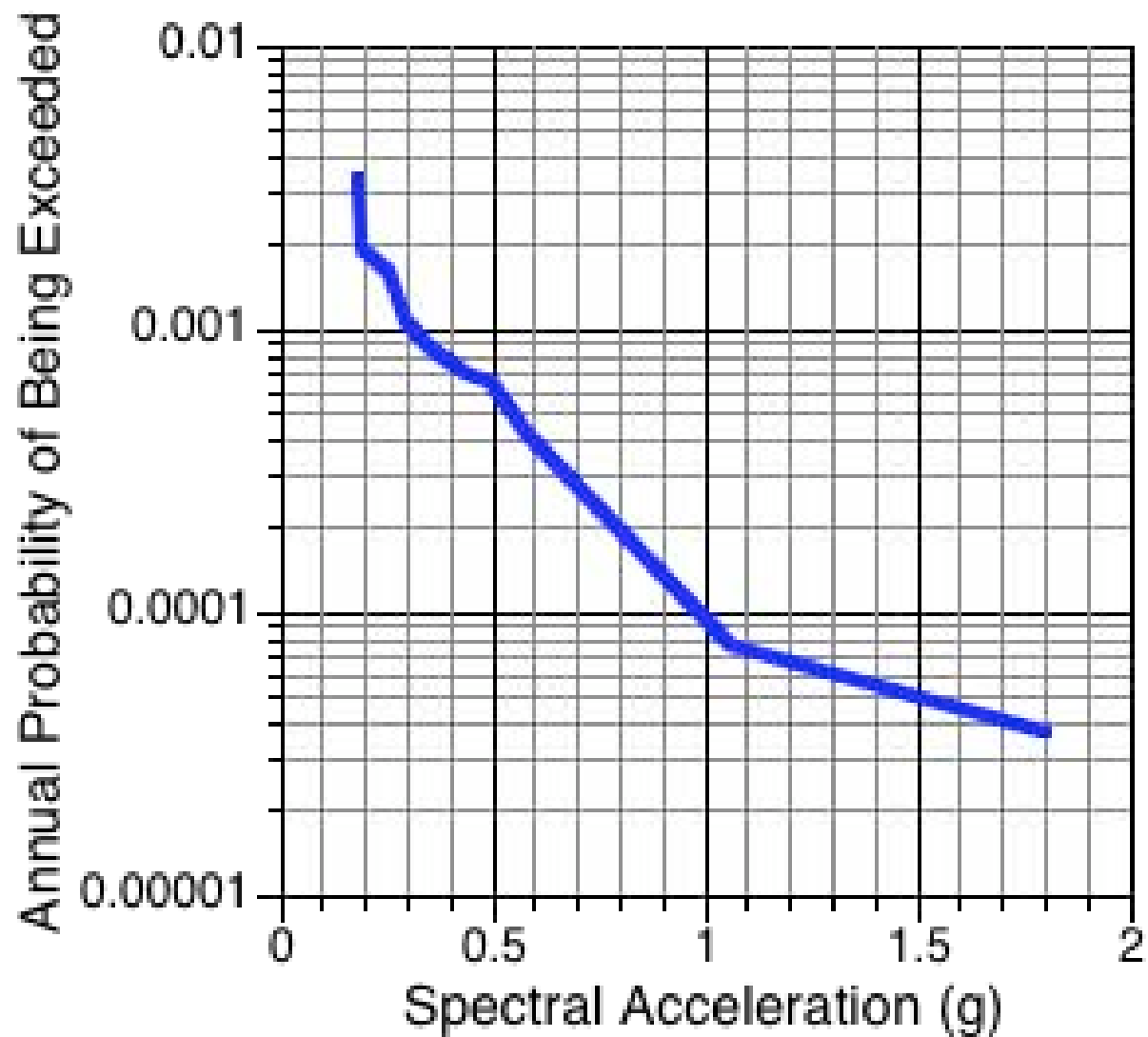
Source	Mag	R (km)	Rate of Sa	Median Sa	Std Dev	$\varepsilon$	P(e)	Sa(g)	Rate
1	6.50	2	0.00022	1.38	0.53	0.5	0.175	1.80	0.000038
1	6.50	2	0.00022	1.38	0.53	-0.5	0.175	1.06	0.000038
1	5.00	2	0.00180	0.58	0.73	0.0	0.197	0.58	0.000355
1	5.00	10	0.00180	0.24	0.73	1.0	0.121	0.49	0.000218
2	5.50	40	0.02216	0.07	0.66	1.5	0.066	0.18	0.001453
2	6.00	40	0.00786	0.10	0.59	1.5	0.066	0.25	0.000516
2	6.50	40	0.00279	0.16	0.52	1.5	0.066	0.35	0.000183
3	7.25	60	0.00170	0.19	0.42	2.0	0.028	0.44	0.000047
3	7.25	60	0.00170	0.19	0.42	1.0	0.121	0.29	0.000206
3	7.25	60	0.00170	0.19	0.42	0.0	0.197	0.19	0.000336

## Rank Scenarios by Ground Motion

Source	Mag	R (km)	$\varepsilon$	Sa(g)	Rate	Hazard
1	6.50	2	0.5	1.80	0.000038	0.000038
1	6.50	2	-0.5	1.06	0.000038	0.000076
1	5.00	10	0.0	0.58	0.000355	0.000432
3	7.25	60	1.0	0.49	0.000218	0.000649
2	6.50	40	1.5	0.44	0.000047	0.000697
3	7.25	60	1.5	0.35	0.000183	0.000880
1	5.00	2	1.5	0.29	0.000206	0.001085
2	6.00	40	2.0	0.25	0.000516	0.001601
3	7.25	60	1.0	0.19	0.000336	0.001937
2	5.50	40	0.0	0.18	0.001453	0.003390



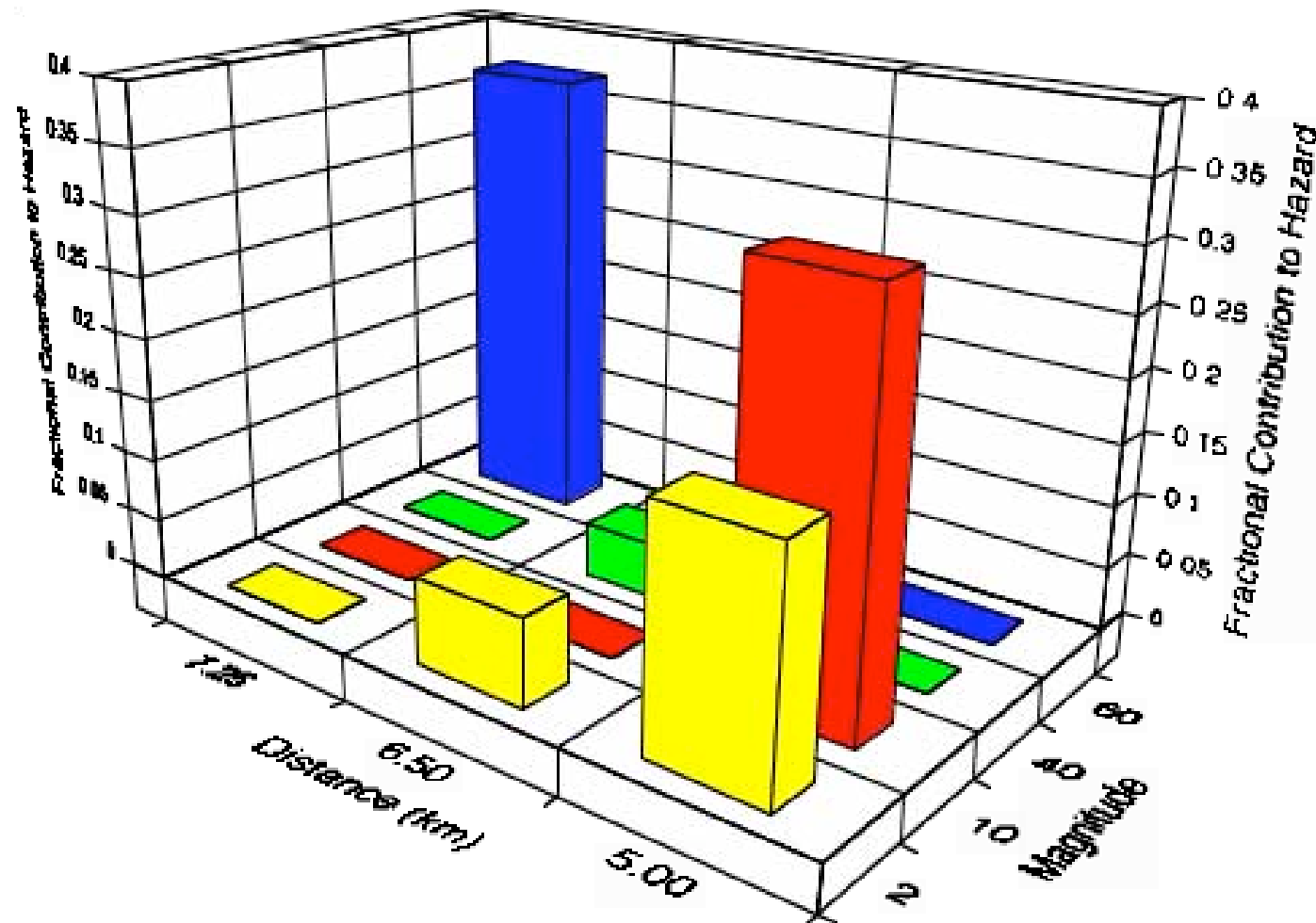
# Hazard Curve



# Deaggregation at $10^{-3}$ Hazard

Source	Mag	R (km)	$\varepsilon$	Sa(g)	Rate	Hazard	Deagg
1	6.50	2	0.5	1.80	0.000038	0.000038	0.035
1	6.50	2	-0.5	1.06	0.000038	0.000076	0.035
1	5.00	10	0.0	0.58	0.000355	0.000432	0.327
3	7.25	60	1.0	0.49	0.000218	0.000649	0.201
2	6.50	40	1.5	0.44	0.000047	0.000697	0.044
3	7.25	60	1.5	0.35	0.000183	0.000880	0.169
1	5.00	2	1.5	0.29	0.000206	0.001085	0.190
2	6.00	40	2.0	0.25	0.000516	0.001601	
3	7.25	60	1.0	0.19	0.000336	0.001937	
2	5.50	40	0.0	0.18	0.001453	0.003390	

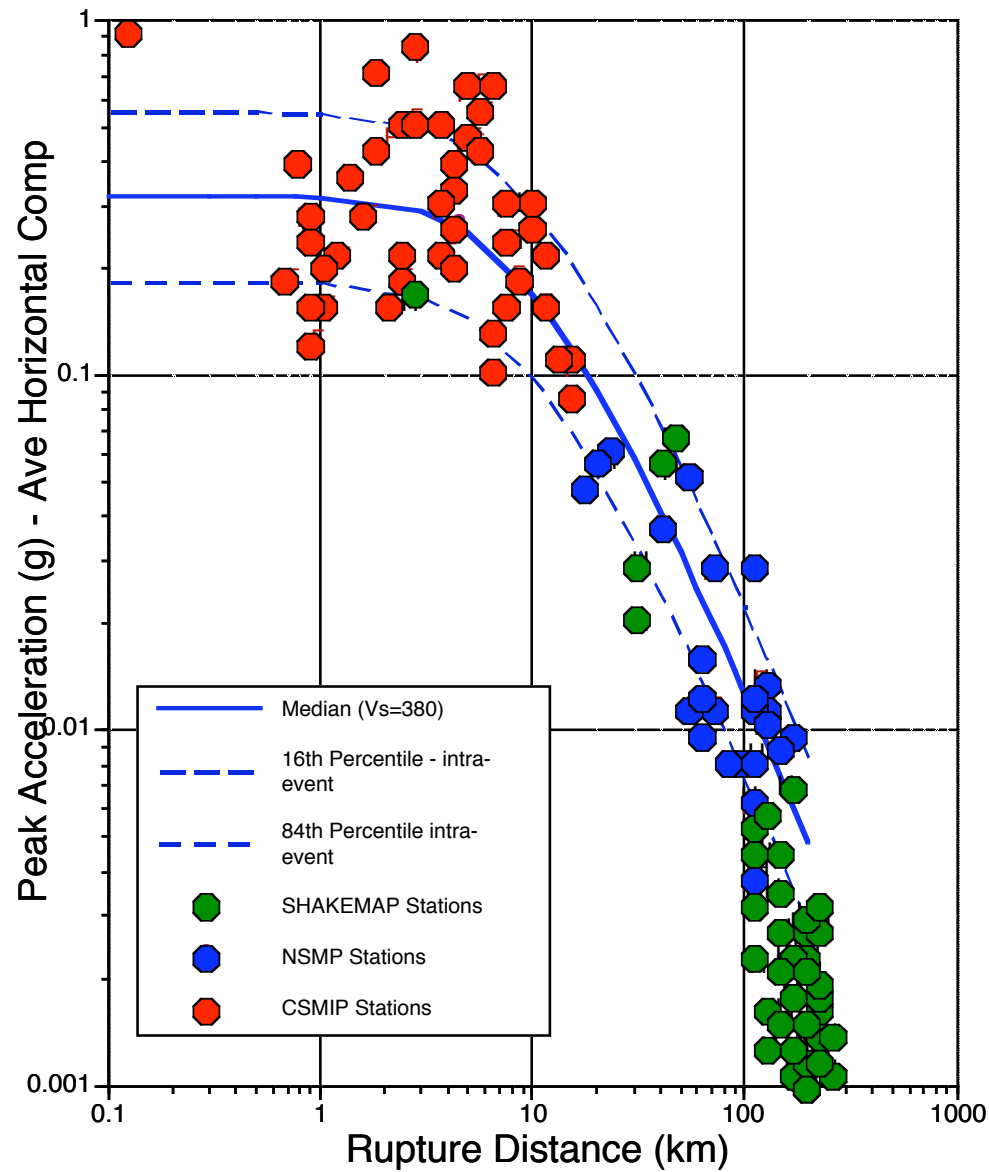
# Group Similar Scenarios for Deaggregation Plots



# Problems with Current Practice

- Major problems are related to the ground motion variability
  - Ignoring the ground motion variability
    - Assumes  $\sigma=0$  for ground motion
    - This is simply wrong. Stop doing it.
  - Applying severe truncation to the ground motion distribution
    - e.g. Distribution truncated at  $+1\sigma$
    - Wishful thinking
    - No empirical basis for truncation at less than  $3\sigma$ .
    - Physical limits of material will truncate the distribution

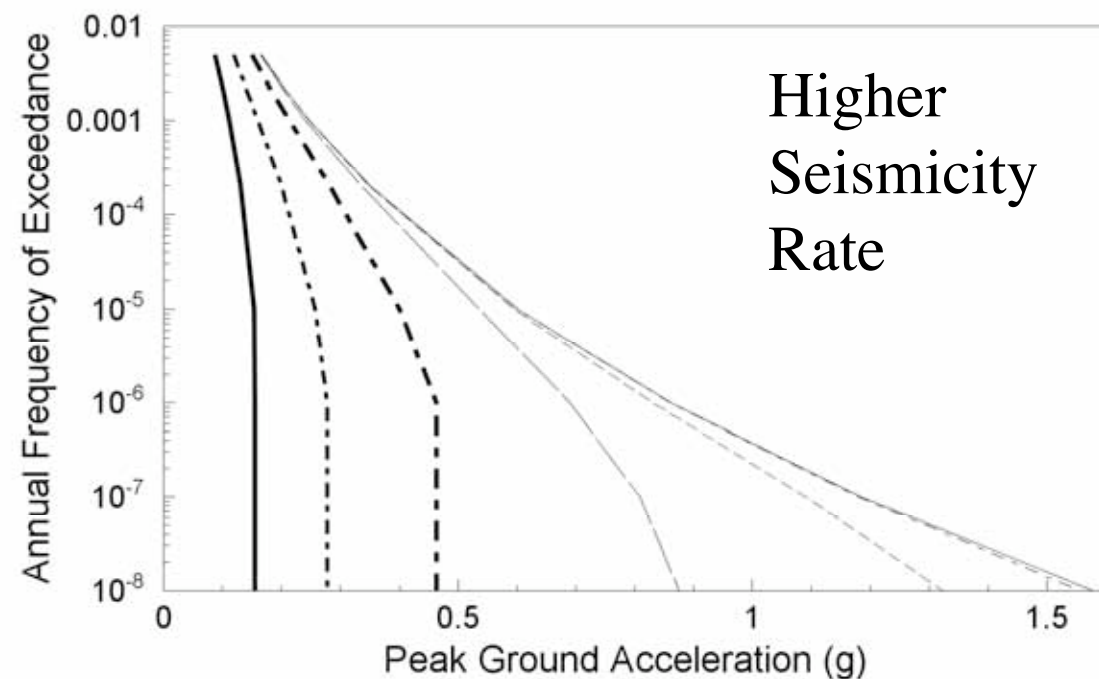
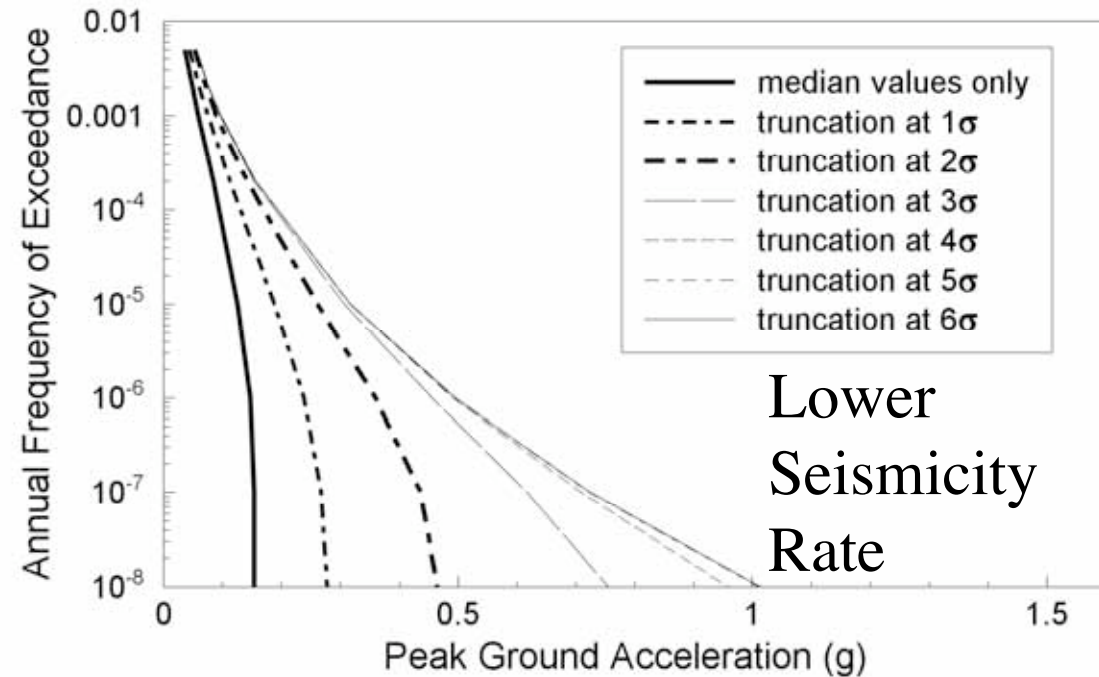
# 2004 Parkfield PGA Attenuation



# Example:

Effect of  
ignoring  $\sigma$ , or  
truncating the  
ground motion  
distribution

(from Bommer, 2004)



# Improvements to Current Practice

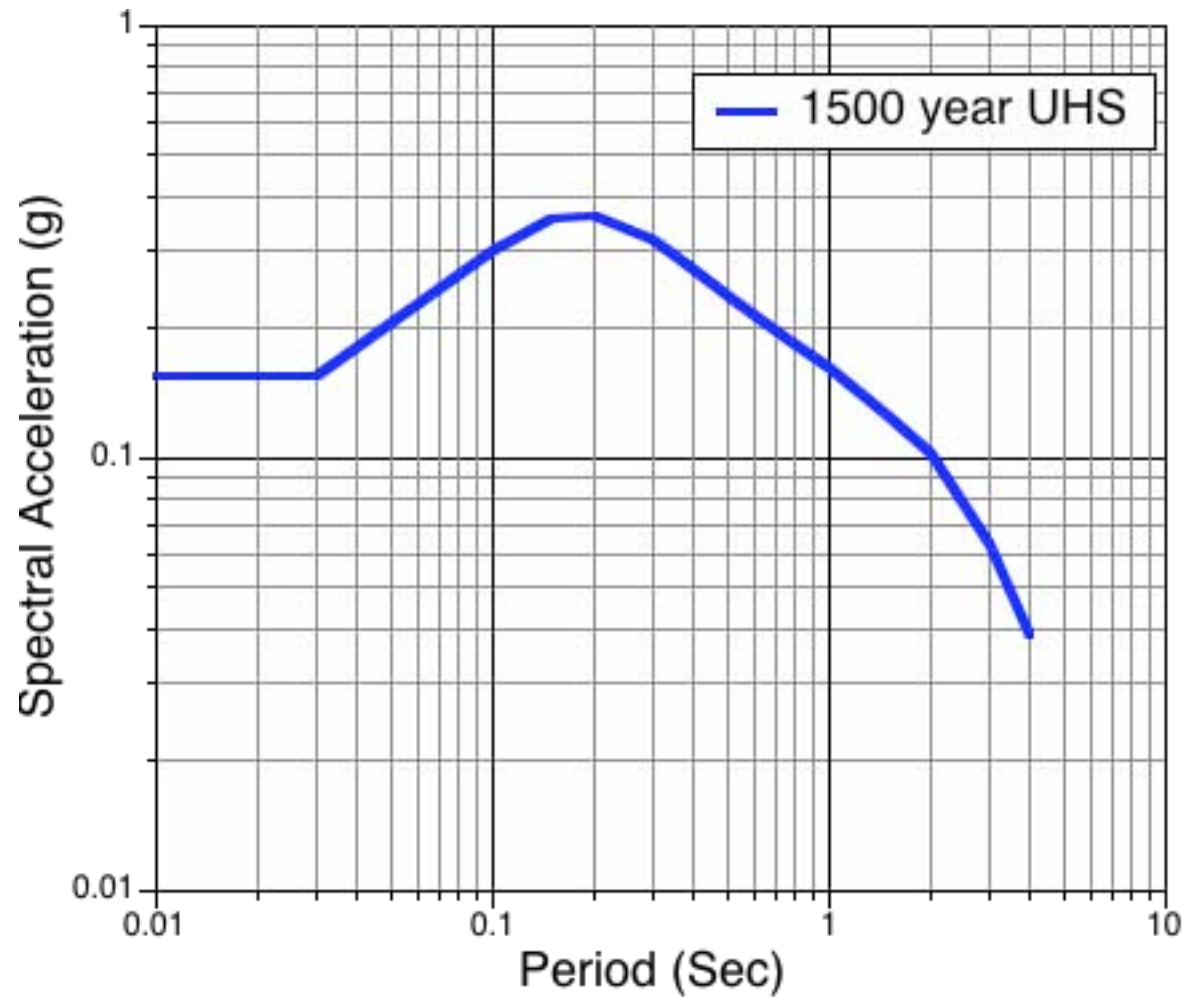
- Scenario Spectra for UHS
- Deaggregation bin size
- Spatial smoothing of seismicity
- Lower bound magnitude for hazard calculation
- Underestimation of epistemic uncertainties

# Uniform Hazard Spectrum

- This is the usual hand-off between the hazard analyst and the engineer. A hazard report includes:
  - UHS at a range of return periods gives the level of the ground motion
  - Deaggregation at several spectral periods for each return period identifies the controlling M,R
- The UHS is not the proper hand-off
  - The UHS is an envelope of the spectra from a suite of earthquakes
  - In addition to the UHS and deaggregation, the hazard analyst needs to provide deterministic scenario spectra that make up the UHS.

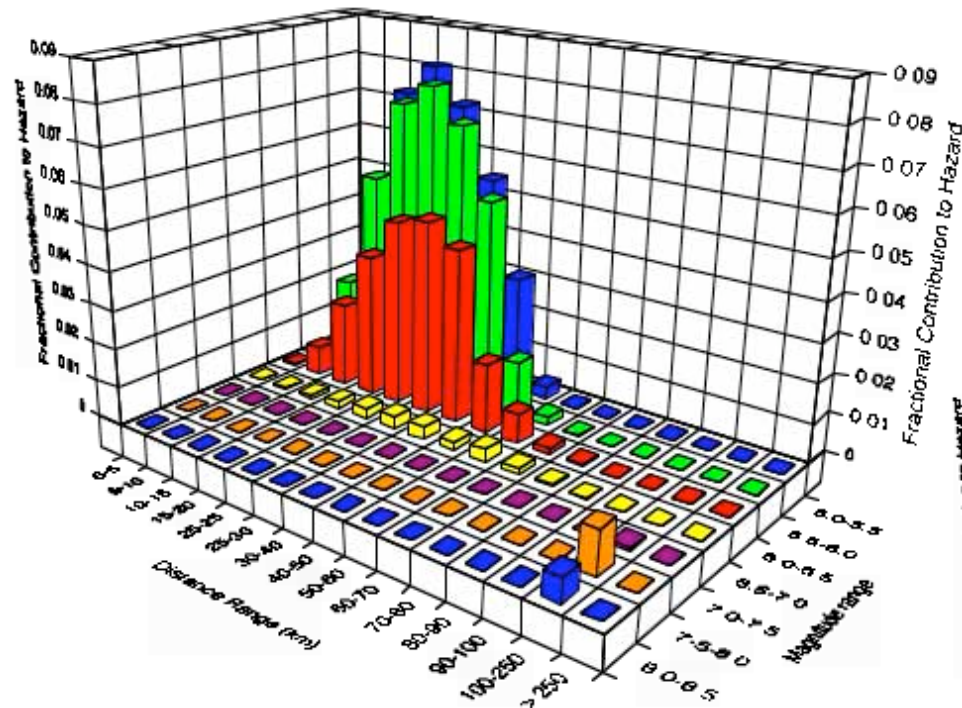


# Crane Valley Dam Example

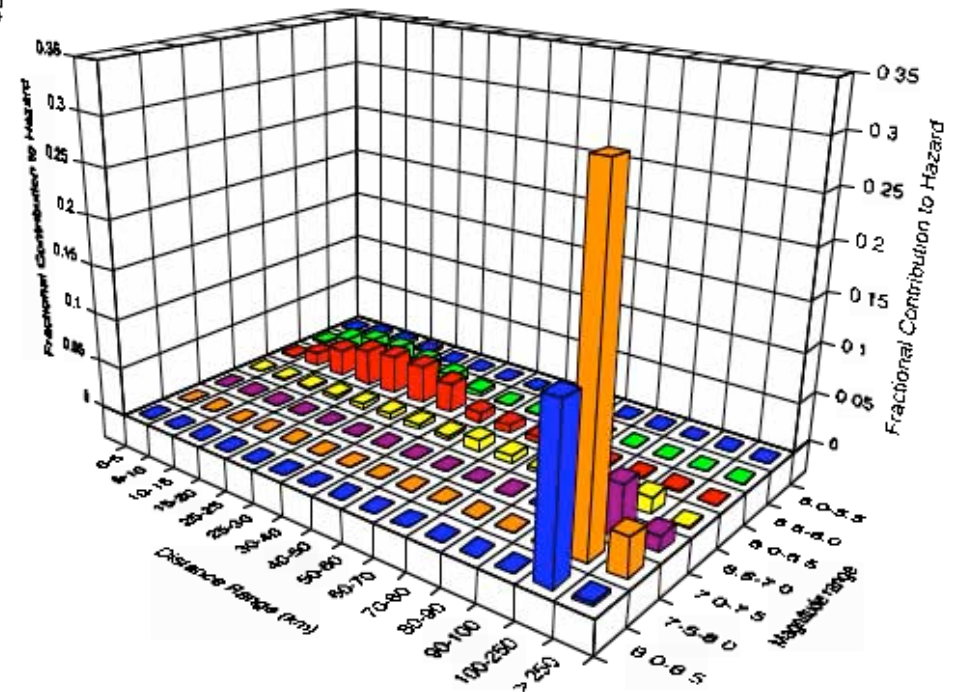


# Deaggregation: 1500 yrs

PGA



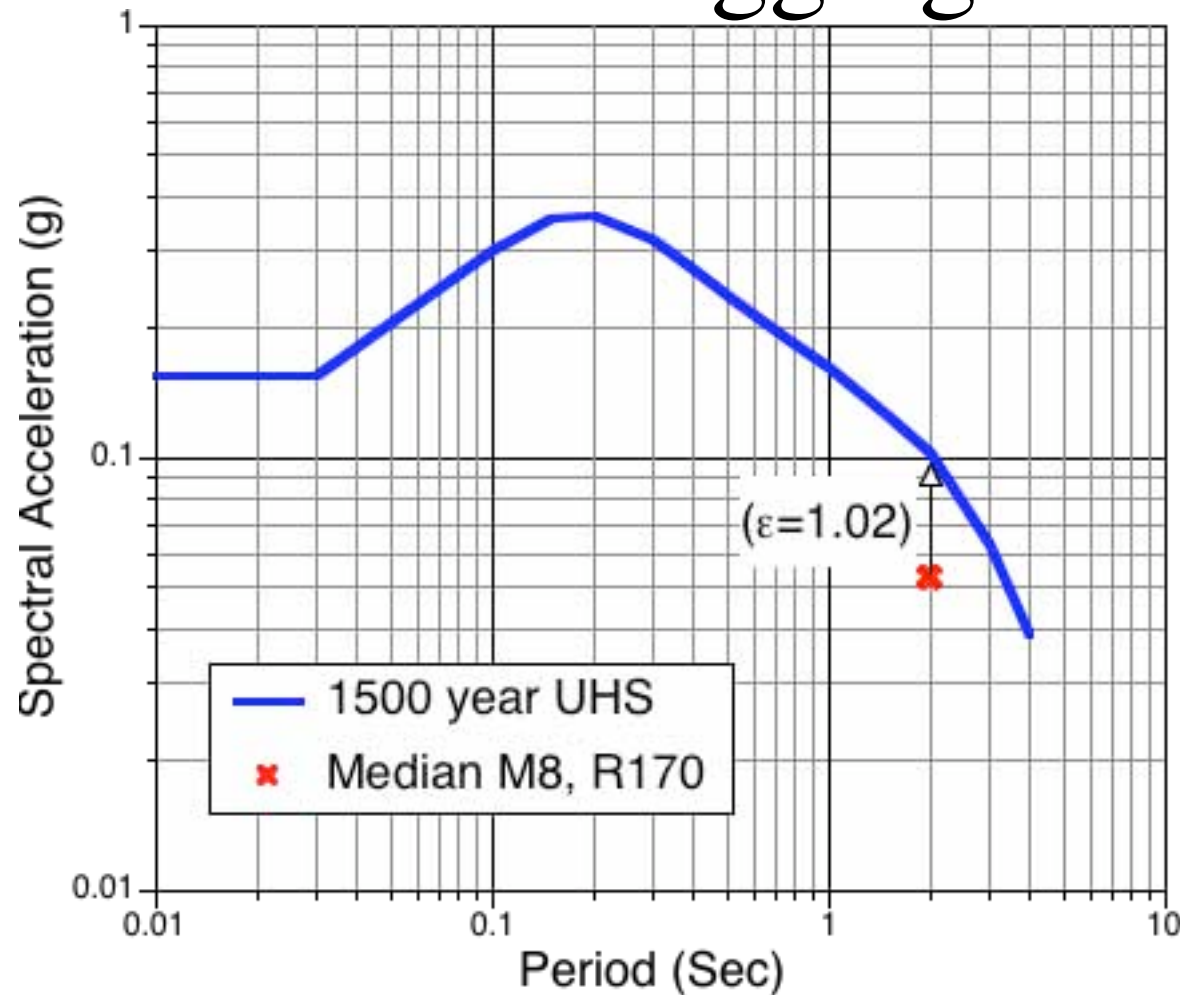
T=2.0 sec



# Controlling Scenarios

- For return period = 1500 years:
  - $S_A(T=0.2)$ :  $M=5.5-6.0$ ,  $R=20-30$  km
  - $S_a(T=2)$ :  $M=7.5-8.0$ ,  $R=170$  km

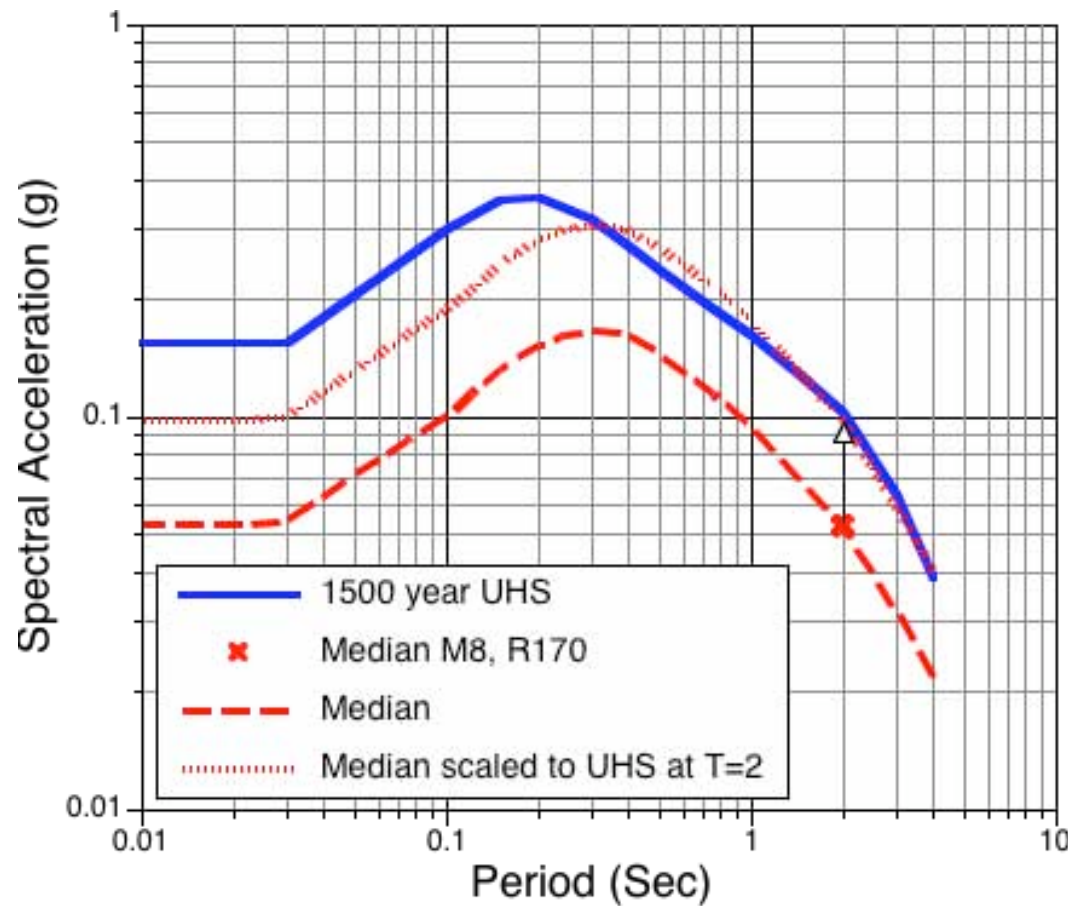
# Scenario Ground Motions from Deaggregation



Find number of standard deviations needed to reach UHS

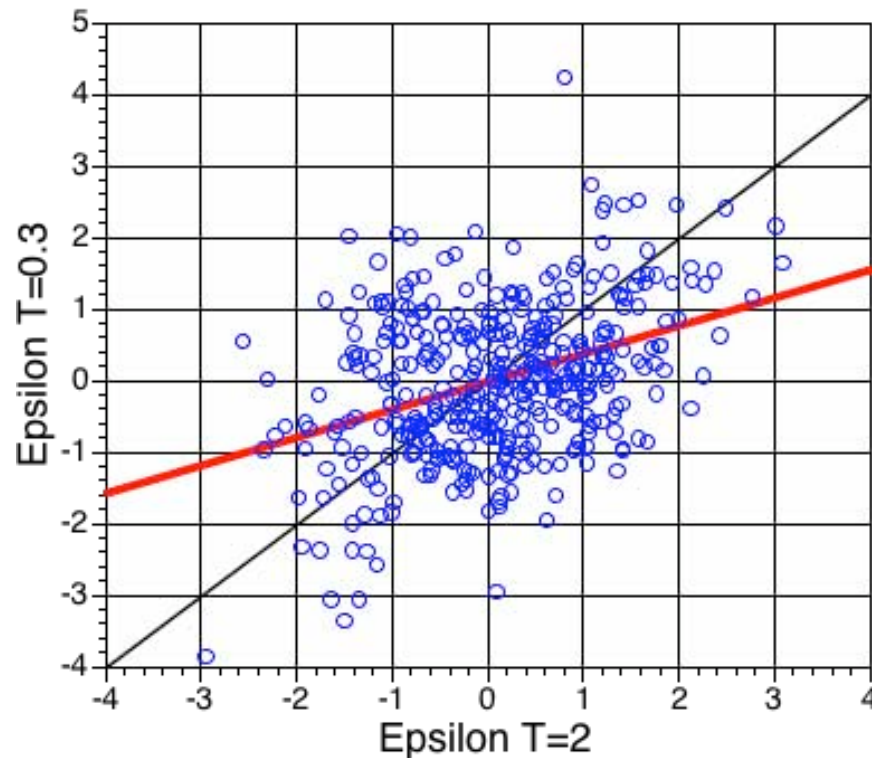
Next,  
Construct the rest of the spectrum

# Construct Scenario Spectrum



- Most common approach uses the median spectral shape, scaled to the UHS
- This approach assumes full correlation between periods

# Expected Spectral Shape

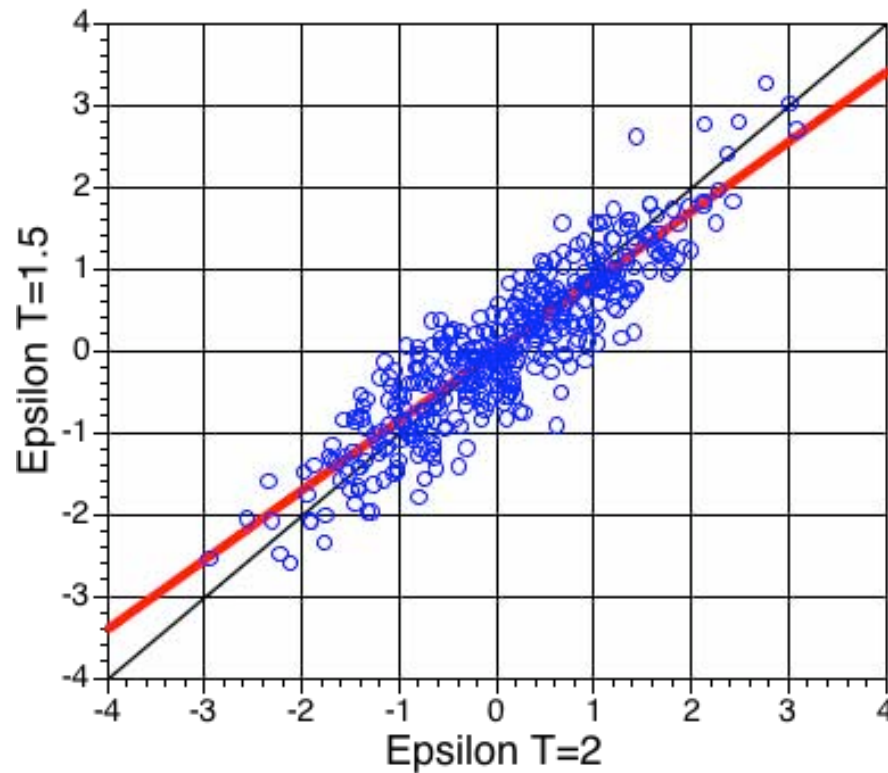


- Depends on the correlation of the epsilon values for different period.
- Find the expected  $\varepsilon(T)$  given  $\varepsilon(T_0)$
- This approach used by Baker and Cornell for scaling time histories

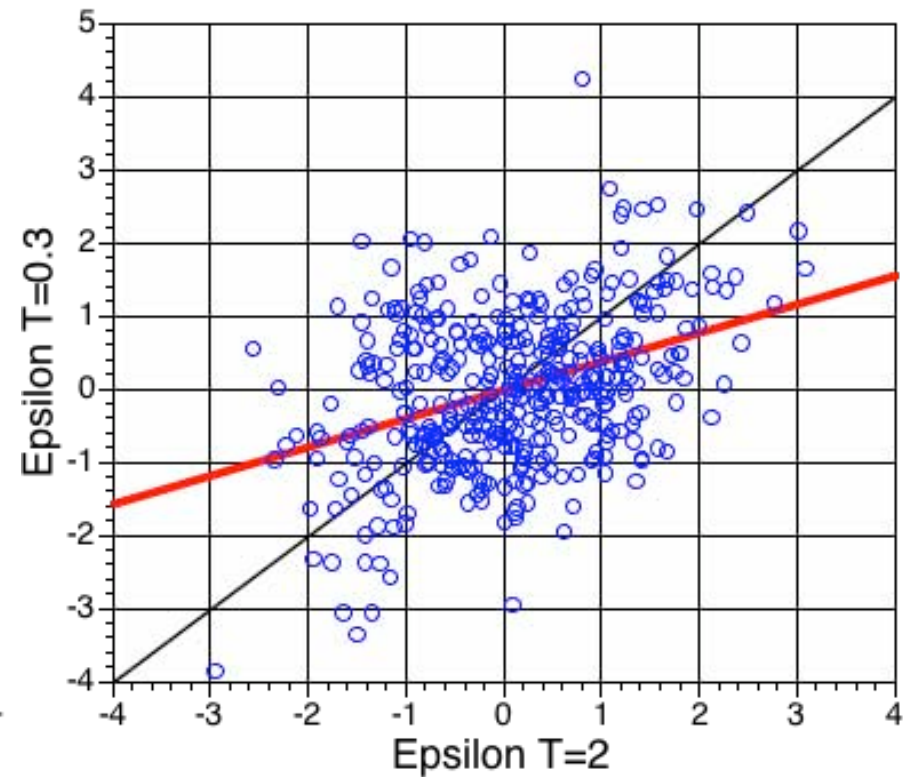


# Correlation of Epsilons is period dependent: $\varepsilon(T)=c\varepsilon(T_o)$

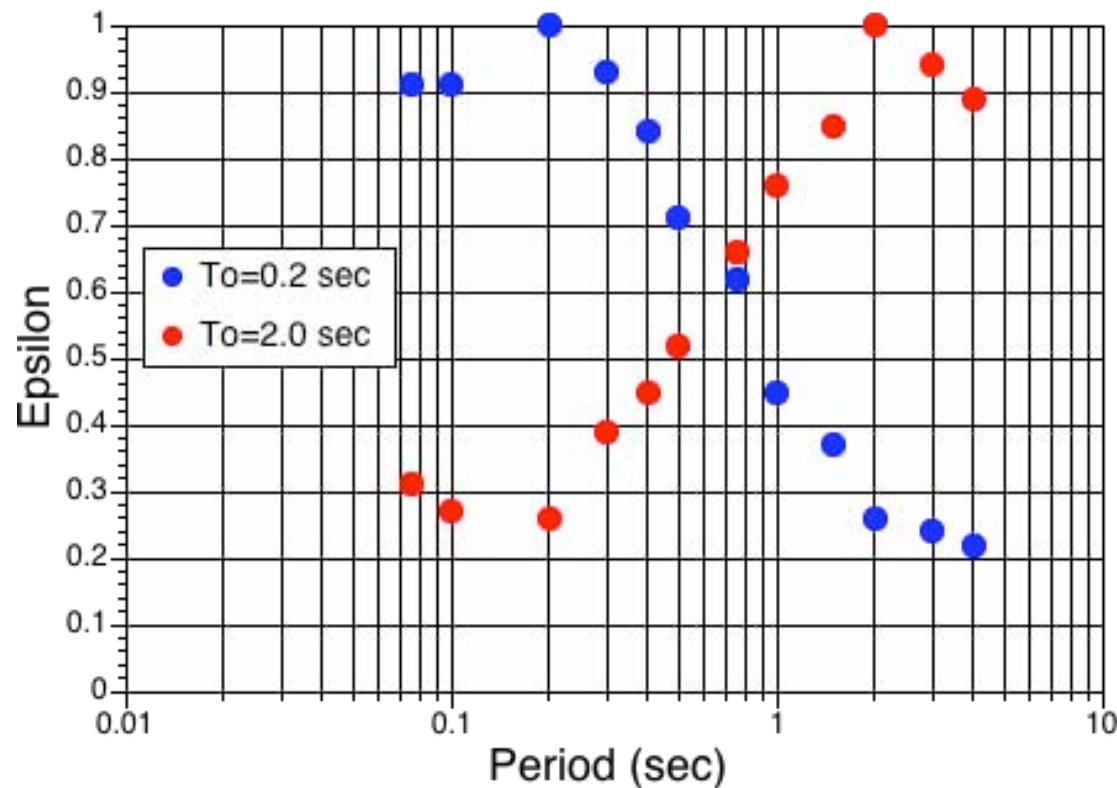
T=1.5



T=0.3



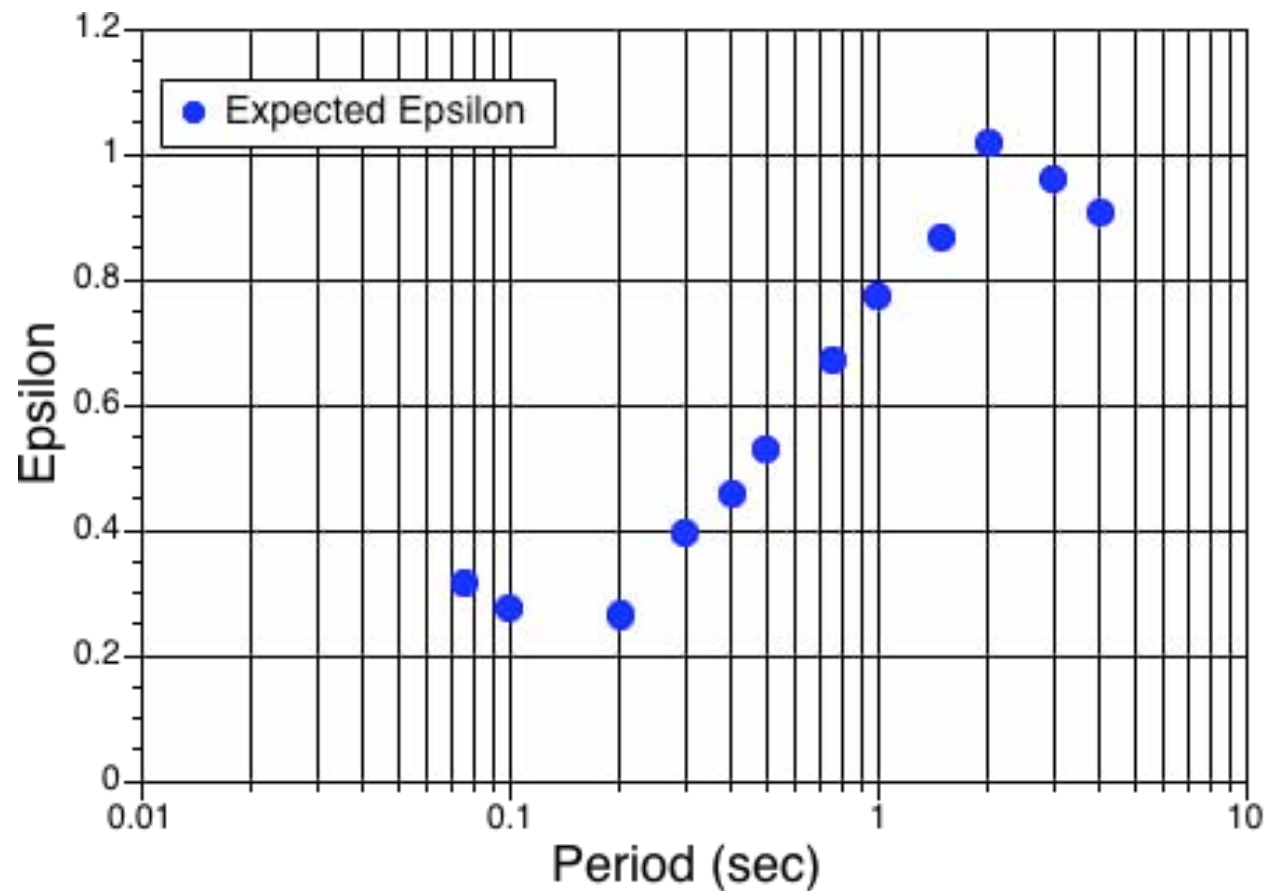
# Correlation of Variability Epsilon(T) with Epsilon ( $T_o$ )



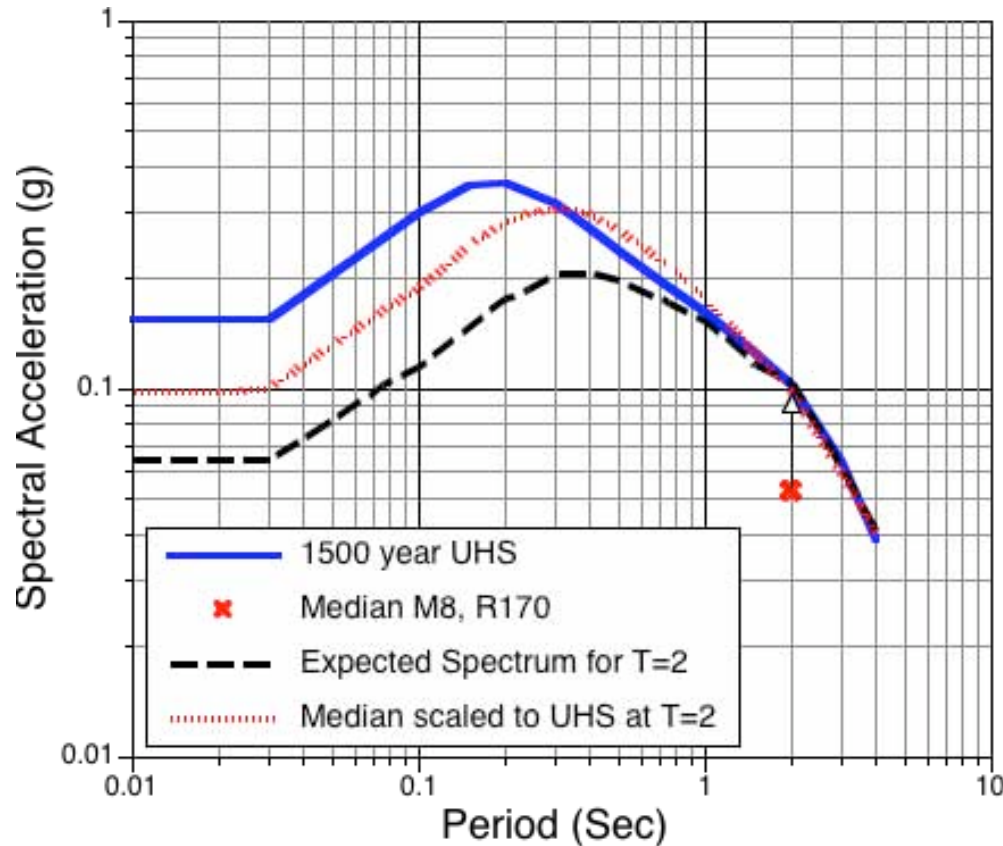
- Correlation decreases away from reference period
- Increase at short period results from nature of  $S_a$



# Mean Epsilon for 1500 yr RP, $T=2$ sec

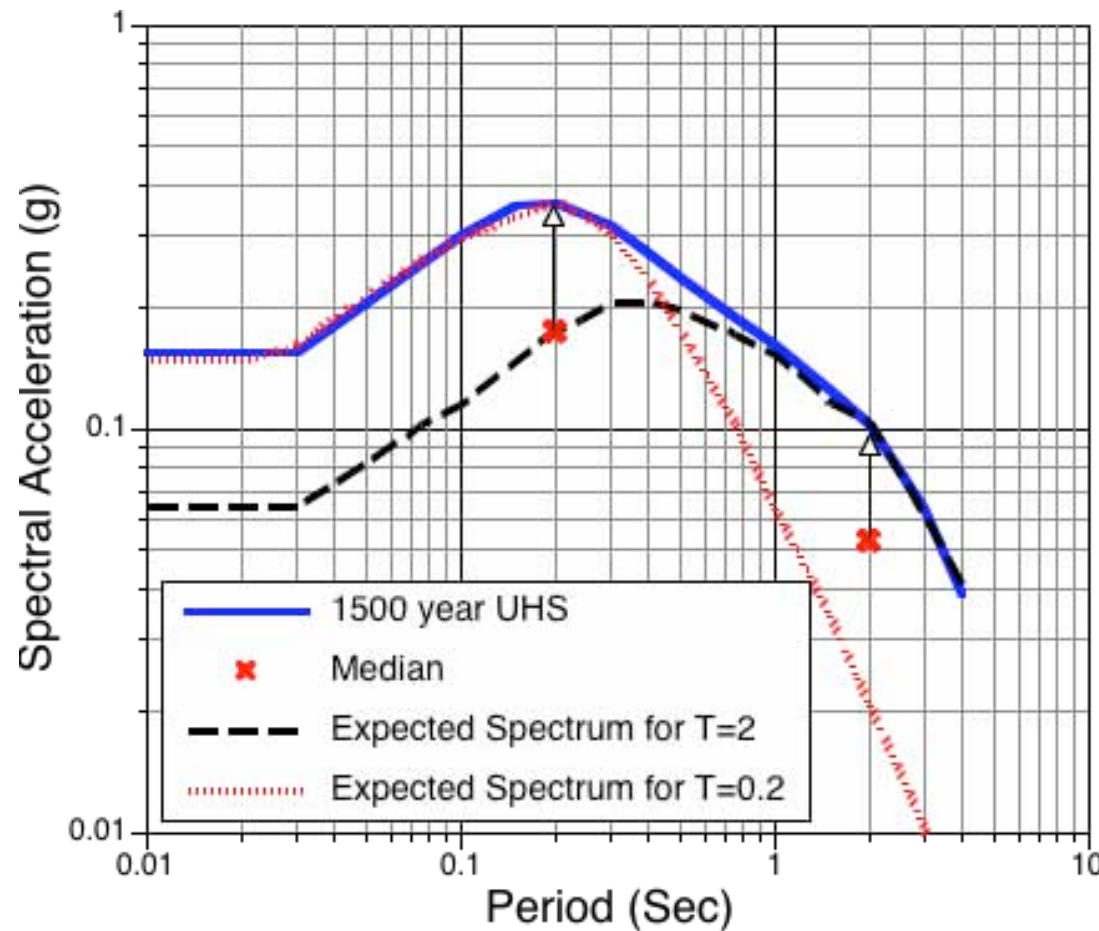


# Scenario Spectrum for 1500 yr RP, T=2 sec



- Realistic scenario
- M=8, R=170 km
- Expected spectrum, if the UHS T=2 sec value occurs
  - $Sa(T) = Sa_{med}(T) \exp(\epsilon(T)\sigma(T))$

# Scenario Spectra for UHS



- Repeat process for other spectral periods
- Develop a suite of deterministic scenarios that comprise the UHS
- Time histories should be matched to the scenarios individually, not to the entire UHS

# Improvements to PHSA Practice

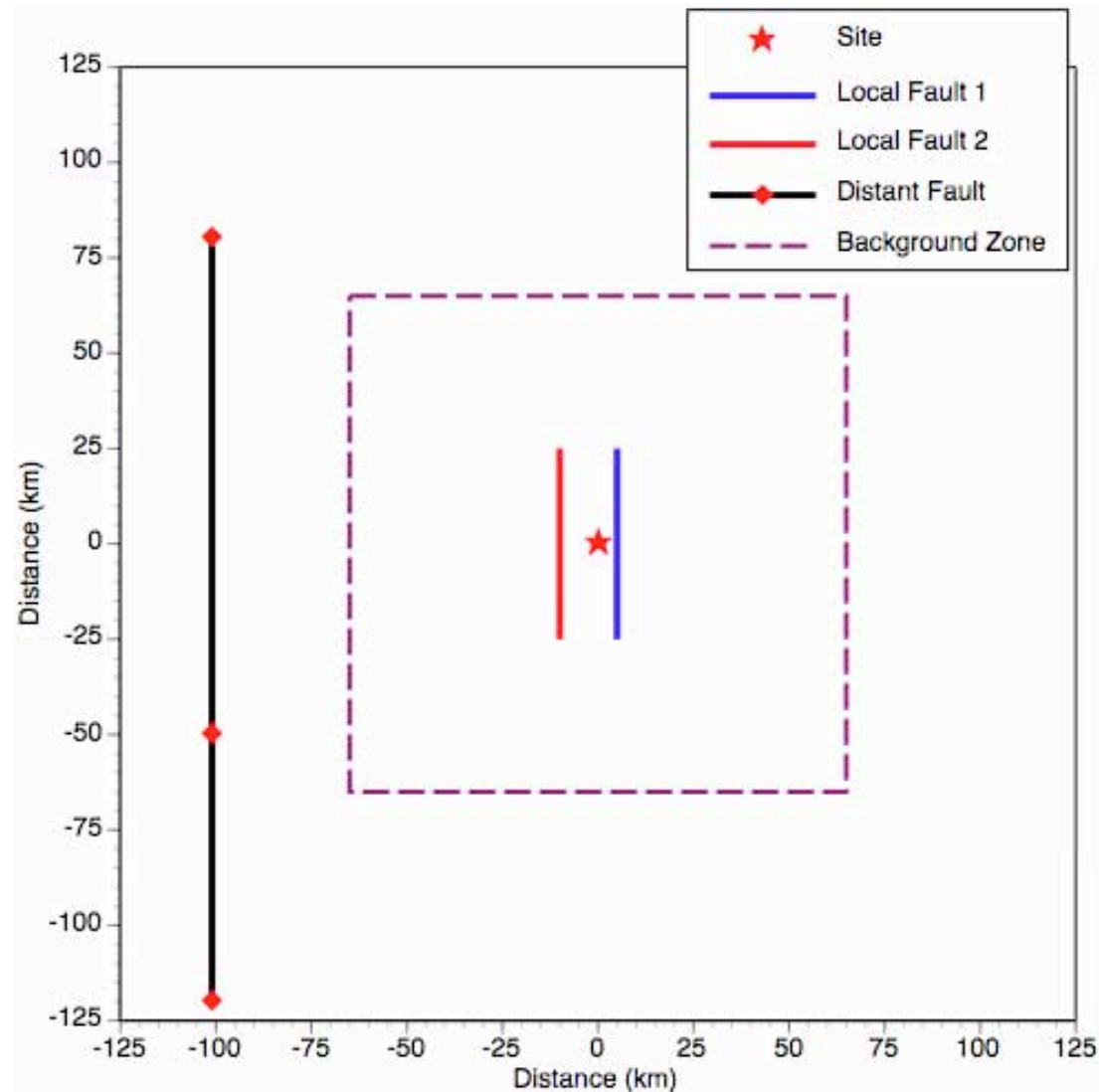
- At the seismology/engineering interface, we need to pass spectra for realistic scenarios that correspond the hazard level
  - This will require suites of scenarios, even if there is a single controlling earthquake
- The decision to envelope the scenarios to reduce the number of engineering analyses required should be made on the engineering side based on the structure, not on the seismology side.

# Deaggregation Bin Size

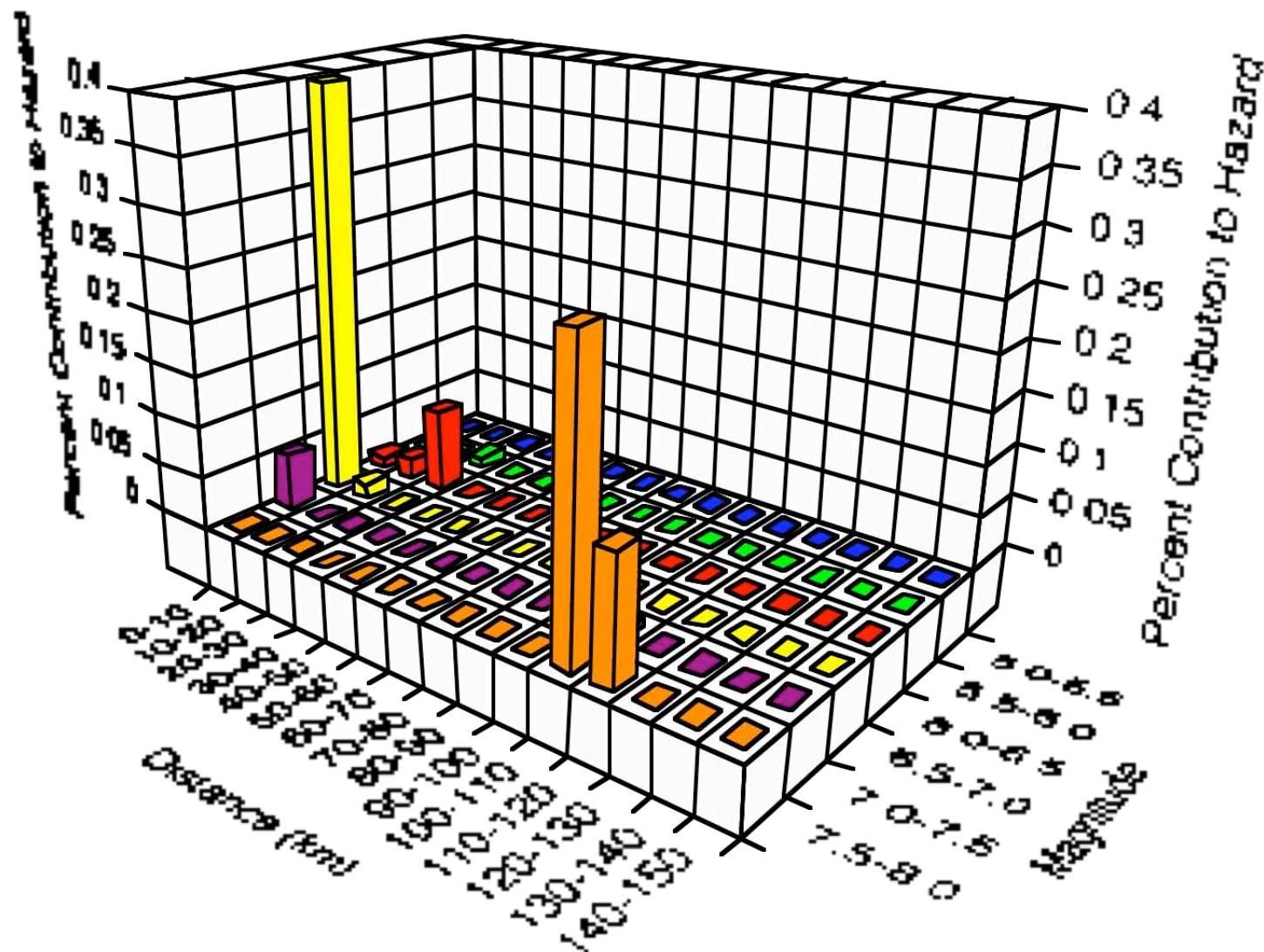
- Common practice to use the mode of the deaggregation in M-R space
  - This avoids the potential problem of finding a non-physical earthquake that can occur using the mean M-R
- Issue:
  - The computed mode depends on the size of the M-R bins used in the deaggregation
  - In current practice, the bin size is set without consideration of the use of the results

# Deaggregation Bin Size

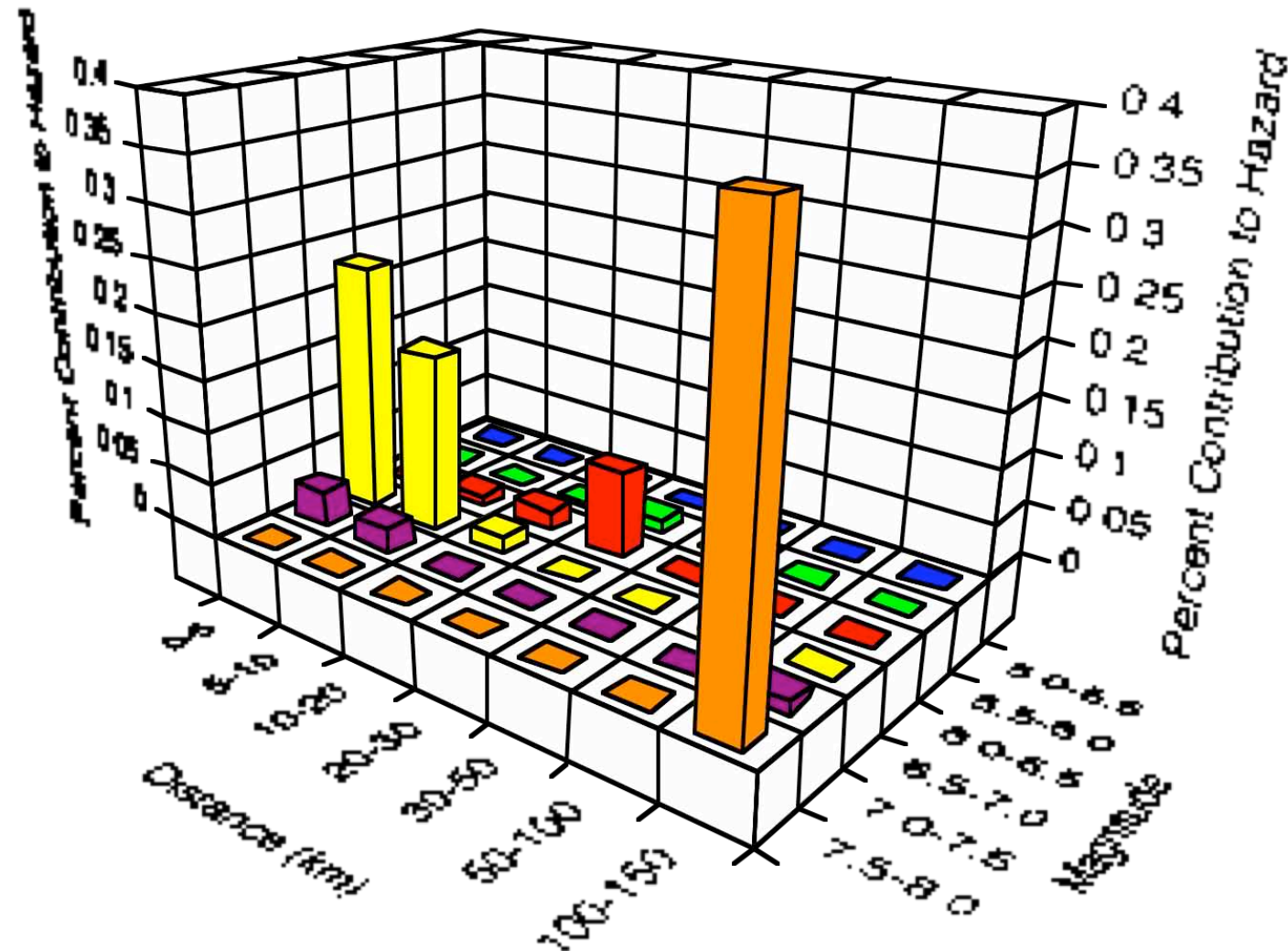
Example  
Source Model



# Deaggregation: Equal Bin Size in R



# Deaggregation: Unequal Bin Size in R





# Deaggregation Bin Size

- Just using equal bin size is not necessarily the best approach for finding the mode
- The best bin size will depend on how the results are to be used
  - e.g. If deaggregation is used for selecting time histories, then unequal spacing in distance should be used.
  - The bin size may be different for different types of structures
  - Should be input from the engineer as to bin sizes.

# Future Developments in PSHA:

- Incorporate site-specific amplification
  - Compute non-linear amplification factors outside of PHSA
- Vector hazard
  - Joint probability of multiple ground motion parameters
- Inelastic spectra
  - Needs vector hazard

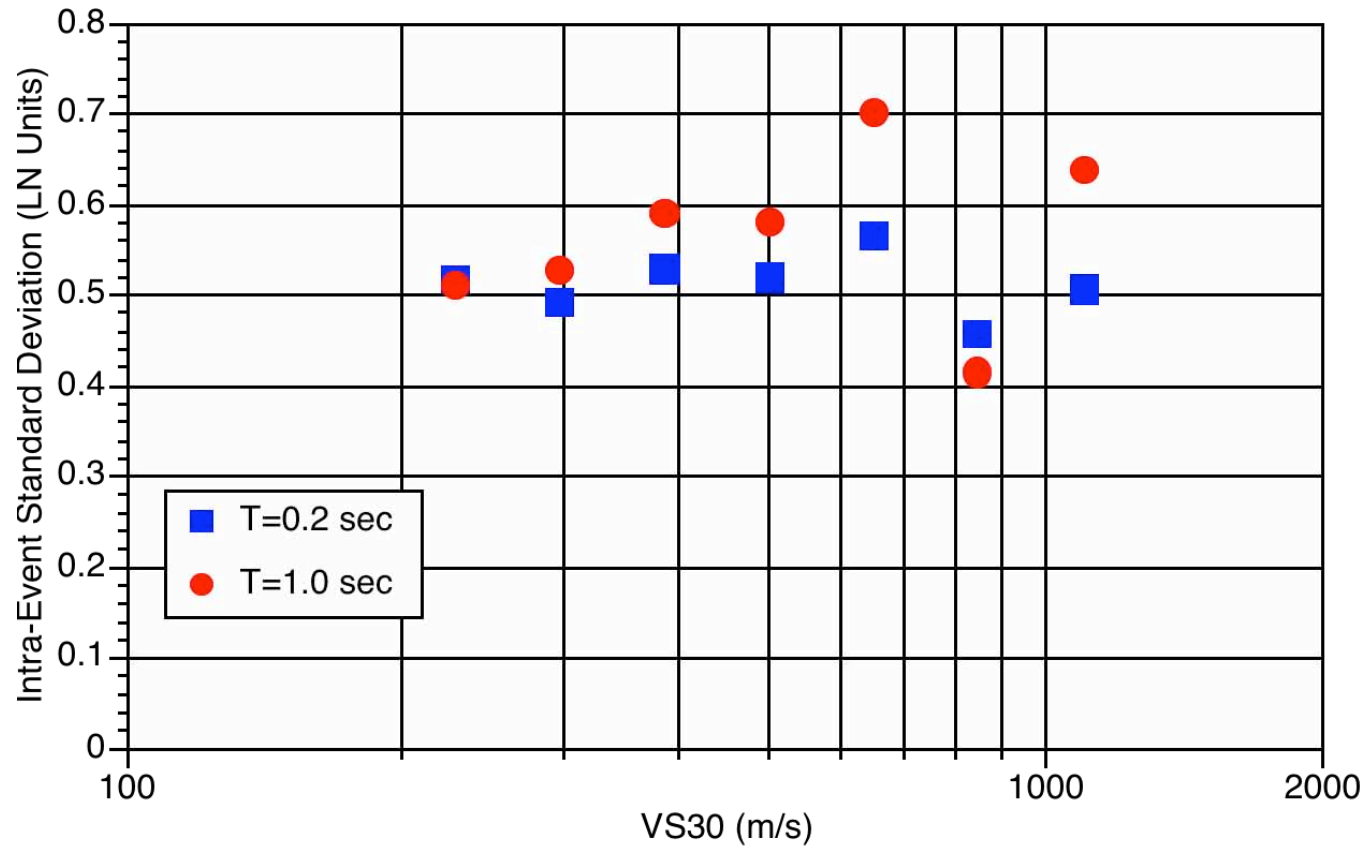
# Incorporating Site-Specific Response in PSHA

- Two Approaches
  - Put the site response inside the hazard integral
    - Requires modification of the PSHA software
  - Compute the hazard using standard PSHA and apply the site response effects in a post-process
- Site Response Models
  - Need to be applicable to all cases relevant to the hazard
    - e.g. range of magnitudes and ground motion levels

# Site Amplification Models

- Median amplification
  - Function of  $M$ ,  $S_a$
- Variability of amplification
  - If linear, then standard deviation on soil will be larger than on rock
  - Not observed

# Standard Deviation from A&S NGA Model by VS30



# Approaches to this Problem

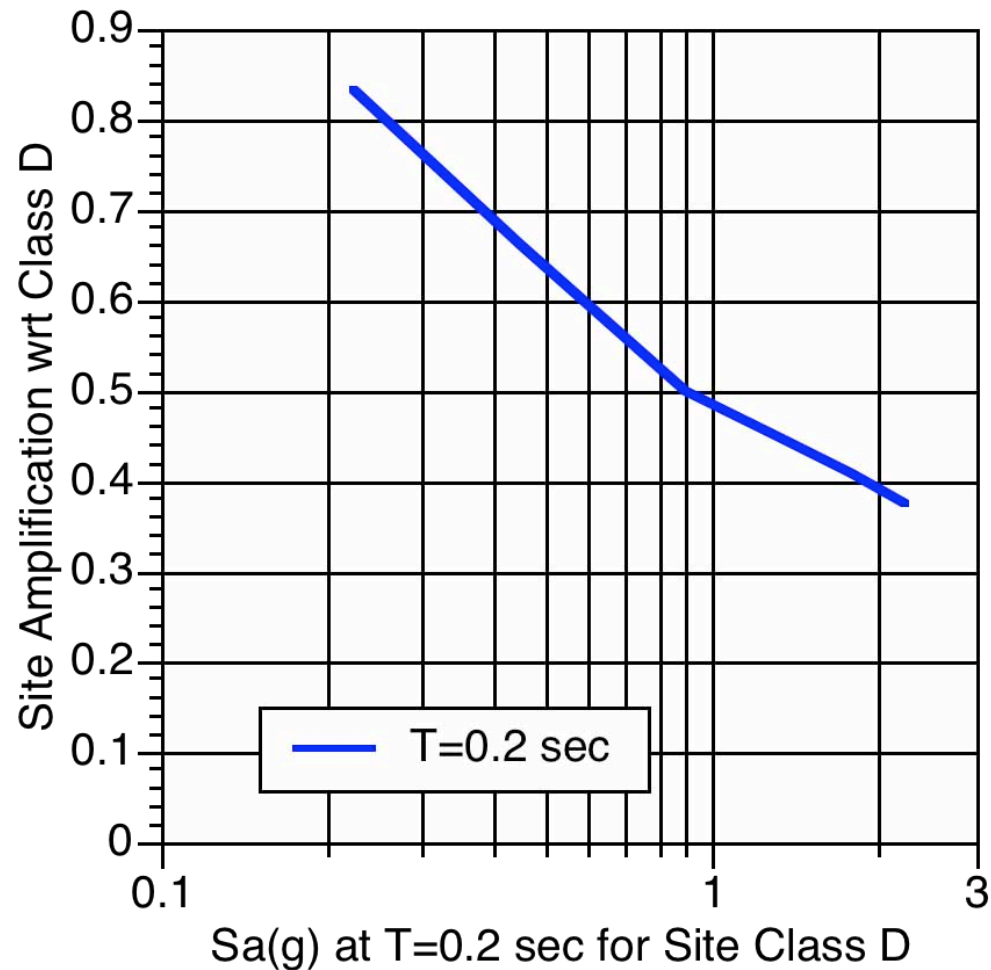
- Use the variability of the amplification and live with the over-estimation of the total variability
- Use only the median amplification and assume that the standard deviation used for the input rock motion is applicable to the soil

# Hazard Example

- Site with engineered fill over class D soil in Los Angeles Region
- Site response computed using SHAKE
  - Magnitudes 6.0, 6.5, 7.0, 7.5
  - PGA (soil D): 0.1, 0.2, 0.4, 0.6, 0.8, 1.0g
  - 7 spectrum compatible time histories used for each case
  - 3 profiles used
  - Total of 504 site response calculations
- Median site amplification used, w/o variability

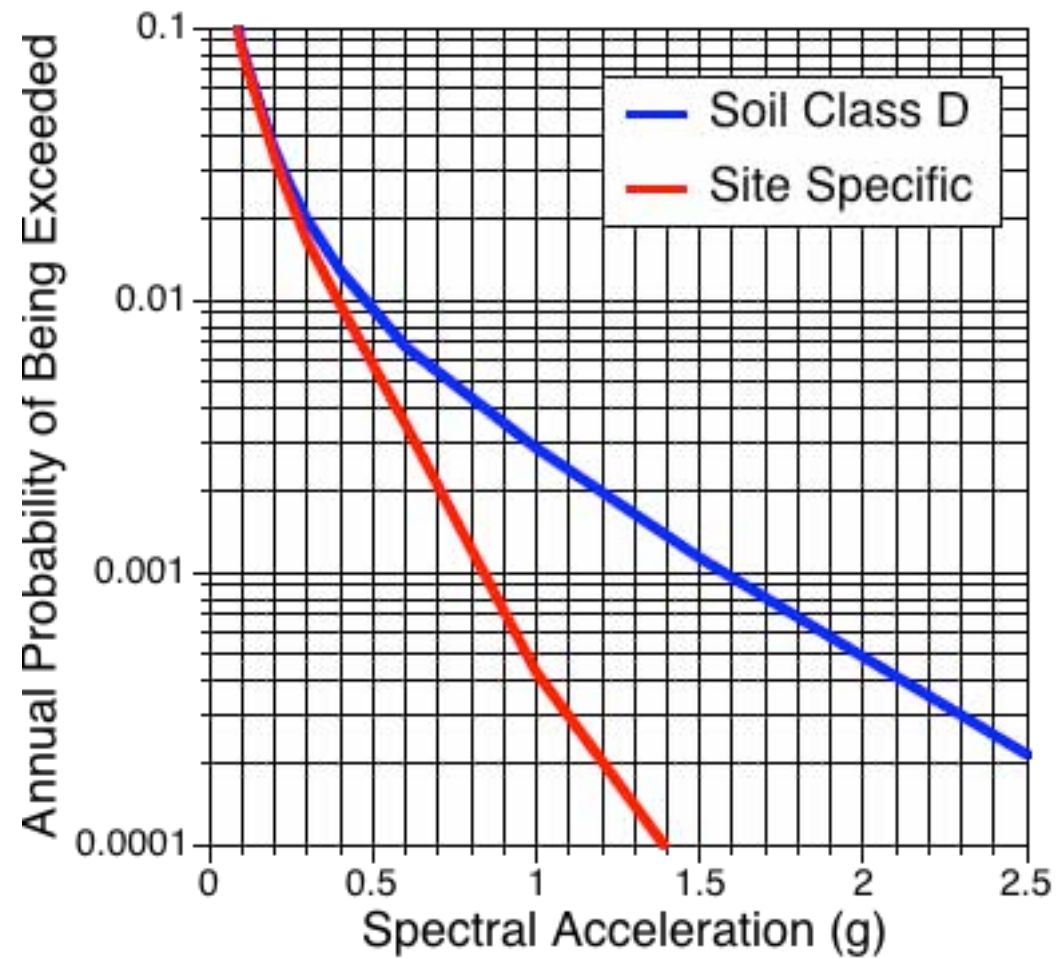
# Median Amplification

## $T=0.2$ sec, $M=6$





# Hazard for $T=0.2$ sec



# Vector Hazard

- Compute the rate of two or more parameters occurring in the same ground motion
  - $S_a(T_1)$  and  $S(2T_1)$
  - PGV and duration,
  - Arias intensity, duration, PGV...
- More likely to lead to significant improvements in predicting structural response than looking for an improved single parameter IM

# Vector Hazard

- Results from vector hazard are best presented in terms of a table of rates of occurrence, rather than as hazard curves.
  - Set bins of the values of each parameter
  - Sum the rates of scenarios that have ground motion values that fall within the bin
- Deaggregation is then conducted for each bin
  - Tables of deterministic scenarios and their rates
- Use of vector hazard is for risk calculations, not development of design ground motions
- Not practical for hazard maps

# PSHA Calculation

- Standard form of hazard

$$v(Sa > z) = \sum_{i=1}^{nSource} N_i(M_{\min}) \int \int_{MR} f_{mi}(M) f_{Ri}(r, M) P(Sa > z | m, R) dR dM$$

- Alternative form (explicit ground motion aleatory variability)

$$v(Sa > z) = \sum_{i=1}^{nSource} N_i(M_{\min}) \int \int \int_{MR\epsilon} f_{mi}(M) f_{Ri}(r, M) f_{\epsilon}(\epsilon) P(Sa > z | m, R, \epsilon) d\epsilon dR dM$$